



STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION

NOTICE TO BIDDERS AND SPECIAL PROVISIONS

FOR CONSTRUCTION ON STATE HIGHWAY IN PLUMAS COUNTY IN AND NEAR QUINCY FROM 0.1 MILE EAST OF OLD COUNTY ROAD TO 0.1 MILE EAST OF ELM STREET

In District 02 On Route 70

Under

Bid book dated April 15, 2013

Standard Specifications dated 2010

Project plans approved March 5, 2013

Standard Plans dated 2010

Identified by
Contract No. 02-4E9804
02-Plu-70-37.5/46.2
Project ID 0212000115

Federal-Aid Project ACSTP-P070(127)E

Electronic Advertising Contract

AADD

SPECIAL NOTICES

- For federal-aid projects, the Department is modifying its DBE program.
- Funding for this project depends on passage of the 2013-2014 FY Budget Act. See section 8-1.04G for special requirements.

CONTRACT NO. 02-4E9804 The special provisions contained herein have been prepared by or under the direction of the following Registered Person.

HIGHWAYS AND TRAFFIC		PROFESSIONAL
REGISTERED CIVIL ENGINEER	35/13 DATE	Denise Fuzeré C69865

ELECTRICAL (HIGHWAY)

REGISTERED ELECTRICAL ENGINEER

3/11/13

Бър. 3-31-13

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REVISED STANDARD SPECIFICATIONS APPLICABLE TO THE 2010 EDITION OF THE STANDARD SPECIFICATIONS	

STANDARD PLANS LIST

The standard plan sheets applicable to this Contract include those listed below. The applicable revised standard plans (RSPs) listed below are included in the project plans.

A10A	Abbreviations (Sheet 1 of 2)
A10B	Abbreviations (Sheet 2 of 2)

A10C Lines and Symbols (Sheet 1 of 3)
A10D Lines and Symbols (Sheet 2 of 3)
A10E Lines and Symbols (Sheet 3 of 3)

A20A Pavement Markers and Traffic Lines, Typical Details
A20B Pavement Markers and Traffic Lines, Typical Details
A20D Pavement Markers and Traffic Lines, Typical Details

RSP A24A Pavement Markings - Arrows

A24B Pavement Markings - Arrows and Symbols
RSP A24C Pavement Markings - Symbols and Numerals

A24D Pavement Markings - Words

RSP A24E Pavement Markings - Words, Limit and Yield Lines

RSP A24F Pavement Markings - Crosswalks

A77A2 Metal Beam Guard Railing - Standard Railing Section (Steel Post with Notched

Wood or Notched Recycled Plastic Block)

A77B1 Metal Beam Guard Railing - Standard Hardware

A77C2 Metal Beam Guard Railing - Steel Post and Notched Wood Block Details

A77C3 Metal Beam Guard Railing - Typical Line Post Embedment and Hinge Point Offset

Details

A77H1 Metal Railing - End Anchor Assembly (Type SFT)

T1A Temporary Crash Cushion, Sand Filled (Unidirectional)T1B Temporary Crash Cushion, Sand Filled (Bidirectional)

T2 Temporary Crash Cushion, Sand Filled (Shoulder Installations)

T3A Temporary Railing (Type K)
T3B Temporary Railing (Type K)

RS1 Roadside Signs, Typical Installation Details No. 1

RS2 Roadside Signs - Wood Post, Typical Installation Details No. 2

RS4 Roadside Signs, Typical Installation Details No. 4

S93 Framing Details for Framed Single Sheet Aluminum Signs, Rectangular Shape

S94 Roadside Framed Single Sheet Aluminum Signs, Rectangular Shape

S95	Roadside Single Sheet Aluminum Signs, Diamond Shape
ES-1A	Electrical Systems (Legend, Notes and Abbreviations)
ES-1B	Electrical Systems (Legend, Notes and Abbreviations)
ES-1C	Electrical Systems (Legend, Notes and Abbreviations)
ES-5A	Electrical Systems (Detectors)
ES-5B	Electrical Systems (Detectors)
ES-5D	Electrical Systems (Curb Termination and Handhole)

CANCELED STANDARD PLANS LIST

The standard plan sheets listed below are canceled and not applicable to this contract.

B3-1	Canceled on April 20, 2012
B3-2	Canceled on April 20, 2012
B3-3	Canceled on April 20, 2012
B3-4	Canceled on April 20, 2012
B3-7	Canceled on April 20, 2012
B3-8	Canceled on April 20, 2012
ES-8	Canceled on January 20, 2012
ES-10	Canceled on July 20, 2012

NOTICE TO BIDDERS

Bids open Wednesday, May 15, 2013

Dated April 15, 2013

General work description: Micro-Surfacing and HMA Overlay

The Department will receive sealed bids for CONSTRUCTION ON STATE HIGHWAY IN PLUMAS COUNTY IN AND NEAR QUINCY FROM 0.1 MILE EAST OF OLD COUNTY ROAD TO 0.1 MILE EAST OF ELM STREET.

District-County-Route-Post Mile: 02-Plu-70-37.5/46.2

Contract No. 02-4E9804

The Contractor must have either a Class A license or one of the following Class C licenses: C-12, C-32.

The DBE Contract goal is 6 percent.

Federal-aid project no.:

ACSTP-P070(127)E

Bids must be on a unit price basis.

Complete the work within 30 working days.

The estimated cost of the project is \$1,680,000.

No prebid meeting is scheduled for this project.

The Department will receive bids until 2:00 p.m. on the bid open date at 1727 30th Street, Bidders' Exchange, MS 26, Sacramento, CA 95816. Bids received after this time will not be accepted. Department staff will direct the bidders to the bid opening.

The Department will open and publicly read the bids at the above location immediately after the specified closing time.

District office addresses are provided in the Standard Specifications.

Present bidders' inquiries to the Department and view the Department's responses at:

http://www.dot.ca.gov/hq/esc/oe/project status/bid inq.html

Questions about alleged patent ambiguity of the plans, specifications, or estimate must be asked before bid opening. After bid opening, the Department does not consider these questions as bid protests.

Submit your bid with bidder's security equal to at least 10 percent of the bid.

Prevailing wages are required on this Contract. The Director of the California Department of Industrial Relations determines the general prevailing wage rates. Obtain the wage rates at the DIR Web site, http://www.dir.ca.gov, or from the Department's Labor Compliance Office of the district in which the work is located.

The federal minimum wage rates for this Contract as determined by the United States Secretary of Labor are available at http://www.dot.ca.gov/hg/esc/oe/federal-wages.

If the minimum wage rates as determined by the United States Secretary of Labor differs from the general prevailing wage rates determined by the Director of the California Department of Industrial Relations for similar classifications of labor, the Contractor and subcontractors must not pay less than the higher wage rate. The Department does not accept lower State wage rates not specifically included in the federal minimum wage determinations. This includes helper, or other classifications based on hours of experience, or any other classification not appearing in the federal wage determinations. Where federal wage determinations do not contain the State wage rate determination otherwise available for use by the Contractor and subcontractors, the Contractor and subcontractors must not pay less than the federal minimum wage rate that most closely approximates the duties of the employees in question.

The Department has made available Notices of Suspension and Proposed Debarment from the Federal Highway Administration. For a copy of the notices, go to http://www.dot.ca.gov/hq/esc/oe/contractor_info. Additional information is provided in the Excluded Parties List System at https://www.epls.gov.

Department of Transportation

D02df

COPY OF BID ITEM LIST

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
1	070030	LEAD COMPLIANCE PLAN	LS	LUMP SUM
2	120090	CONSTRUCTION AREA SIGNS	LS	LUMP SUM
3	120100	TRAFFIC CONTROL SYSTEM	LS	LUMP SUM
4	128652	PORTABLE CHANGEABLE MESSAGE SIGN (LS)	LS	LUMP SUM
5	130100	JOB SITE MANAGEMENT	LS	LUMP SUM
6	130200	PREPARE WATER POLLUTION CONTROL PROGRAM	LS	LUMP SUM
7	141103	REMOVE YELLOW THERMOPLASTIC TRAFFIC STRIPE (HAZARDOUS WASTE)	LF	45,300
8	141104	REMOVE YELLOW THERMOPLASTIC PAVEMENT MARKING	SQFT	460
9	141120	(HAZARDOUS WASTE) TREATED WOOD WASTE	LB	18,000
10	150714	REMOVE THERMOPLASTIC TRAFFIC STRIPE	LF	123,000
11	150715	REMOVE THERMOPLASTIC PAVEMENT MARKING	SQFT	9,670
12	151572	RECONSTRUCT METAL BEAM GUARD RAILING	LF	1,260
13	152469	ADJUST UTILITY COVER TO GRADE	EA	130
14	152500	ADJUST METAL BEAM GUARD RAILING	LF	130
15	153103	COLD PLANE ASPHALT CONCRETE PAVEMENT	SQYD	16,000
16	378000	MICRO-SURFACING	TON	3,970
17	390010	PREPAVING PROFILOGRAPH	LS	LUMP SUM
18	390020	PREPAVING GRINDING DAY	EA	1
19	390095	REPLACE ASPHALT CONCRETE SURFACING	CY	590
20	390132	HOT MIX ASPHALT (TYPE A)	TON	2,380

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
21	397005	TACK COAT	TON	11
22	839581	END ANCHOR ASSEMBLY (TYPE SFT)	EA	2
23	840515	THERMOPLASTIC PAVEMENT MARKING	SQFT	11,200
24	840560	THERMOPLASTIC TRAFFIC STRIPE (SPRAYABLE)	LF	168,000
25	850122	PAVEMENT MARKER (RETROREFLECTIVE- RECESSED)	EA	6,020
26	860090	MAINTAINING EXISTING TRAFFIC MANAGEMENT SYSTEM ELEMENTS DURING CONSTRUCTION	LS	LUMP SUM
27	861502	MODIFY SIGNAL	LS	LUMP SUM
28	025589	MODIFY TRAFFIC COUNT STATION	LS	LUMP SUM

SPECIAL PROVISIONS

DIVISION I GENERAL PROVISIONS 1 GENERAL

Add to section 1-1.01:

Bid Items and Applicable Sections

Item code	Item description	Applicable section
025589	Modify Traffic Count Station	86

Add to section 1-1.09:

This project is in a freeze-thaw area.

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2 BIDDING

Add to section 2-1.06B:

The Department makes the following supplemental project information available:

Supplemental Project Information

Means	Description
Included in the Information Handout	PLAC - Permit Condition Responsibility Summary
	PLAC - Encroachment Permit

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5 CONTROL OF WORK

During the progress of the work under this Contract, work under the following contracts may be in progress at or near the job site of this Contract:

Coincident or Adjacent Contracts

Contract no.	Co-Rte-PM	City	Type of work
Encroachment Permit	PLU-70-42.9/43.68	Quincy	Replace Sewer Line and Manhole Covers

Add to section 5-1.32:

The following locations meet environmental compliance for parking and stockpiling:

Co-Route	(i.e., NB, SE		Description of Area and Allowed Use						
Plu-70	37.75	EB	Gravel area inside of turn						
Plu-70	Plu-70 38.1 EB		Narrow gravel pull-out used for dirt road access						
Plu-70	Plu-70 40.0 WB		Large gravel pull-out with dirt road access						

Add to section 5-1.36D:

Notify the Engineer a minimum of 7 days prior to utility adjustment activities for the facilities shown in the following table:

Utilities To Be Adjusted During Construction

Utility	Location
East Quincy Service District - Sewer	Plu-70-37.5-46.22
East Quincy Service District - Water	Plu-70-37.5-46.22
Quincy Community Services District - Water	Plu-70-37.5-46.22
Quincy Community Services District - Sewer	Plu-70-37.5-46.22

Complete utility information is not shown on the project plans. Utility location activities must be concurrent with construction activities. Make arrangements with the utility owners to coordinate utility location activities. Mechanically actuated excavation must not be performed within 24 inches horizontally of the exterior surface of the subsurface facilities.

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7 LEGAL RELATIONS AND RESPONSIBILITY TO THE PUBLIC

Replace section 7-1.02K(6)(j)(iii) with:

7-1.02K(6)(j)(iii) Earth Material Containing Lead

Section 7-1.02K(6)(j)(iii) includes specifications for handling, removing, and disposing of earth material containing lead.

Submit a lead compliance plan.

Lead is present in earth material on the job site. The average lead concentrations are below 1,000 mg/kg total lead and below 5 mg/L soluble lead. The material on the job site:

- 1. Is not a hazardous waste
- 2. Does not require disposal at a permitted landfill or solid waste disposal facility

Lead is typically found within the top 2 feet of material in unpaved areas of the highway. Reuse all of the excavated material on the right-of-way.

Handle the material under all applicable laws, rules, and regulations, including those of the following agencies:

- 1. Cal/OSHA
- 2. CA RWQCB, Region 5—Central Valley
- 3. CA Department of Toxic Substances Control

Add to section 7-1.03:

If a height differential of more than 0.04 foot is created by construction activities at joints transverse to the direction of traffic on the traveled way or shoulder subject to public traffic, construct temporary tapers with slopes (horizontal:vertical) in conformance with the following table:

Temporary Tapers

	Slope				
Height differential		Taper use of more			
	Taper use of 14 days or less	than 14 days			
Greater than 0.10 foot	100:1 or flatter	200:1 or flatter			
0.04 to 0.10 foot	70:1 or flatter	70:1 or flatter			

For temporary tapers on existing asphalt concrete or concrete pavement, construct tapers with HMA that complies with section 39-1.15B. HMA spreading and compacting equipment must comply with section 39-1.10.

Grind existing surfaces to accommodate a minimum taper thickness of 0.10 foot when either:

- 1. HMA material such as rubberized, polymer modified or open graded is unsuitable for raking to a maximum 0.02 foot thickness at the conform
- 2. Temporary taper will be in place for more than 14 days

For temporary tapers on bridge decks or approach slabs, construct temporary tapers with polyester concrete that complies with section 15-5.06B.

The completed temporary taper surface must be uniform and must not vary from the lower edge of a 12-foot straightedge more than 0.02 foot when laid upon its surface in directions parallel and perpendicular to traffic. If authorized, you may use alternative materials or methods to produce the required taper. This temporary taper slope must be used instead of any other temporary transverse joint taper slope described in the Contract.

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8 PROSECUTION AND PROGRESS

Replace section 8-1.04G with:

8-1.04G Potential Budget Impasse Start

The 1st paragraph of section 8-1.04B does not apply.

Within 10 days after receiving notice that the Contract has been approved by the Attorney General or the attorney appointed and authorized to represent the Department or within 5 business days after the 2013-2014 FY Budget Act becomes law, whichever occurs later, submit a request for authorization to start job site activities. The request must include:

- 1. CPM baseline schedule
- 2. Date you plan to start job site activities

The Department does not allow changes to the request after it is authorized.

Except for measuring controlling field dimensions and locating utilities, do not start job site activities until your WPCP or SWPPP, whichever applies, is received and authorized and the following submittals are received:

1. Notice of Materials To Be Used

2. Contingency plan for reopening closures to public traffic

If you obtain authorization to start job site activities for the date you requested, start job site activities on the requested date. If you fail to submit a request for authorization to start job site activities as specified or if the request is not authorized, start job site activities within 15 days after Contract approval or 5 business days after the 2013-2014 FY Budget Act becomes law, whichever occurs later, except as specified below for the case of funds becoming available in the 2012-2013 FY Budget Act. Start work before August 1, 2013.

Do not start work before July 1, 2013 unless:

- 1. Funds become available in the 2012-2013 FY Budget Act
- 2. The Engineer and you agree on a start date after Contract approval

If the 2013-2014 FY Budget Act does not become law by August 1, 2013 and no work on the Contract has been performed, you may terminate the Contract at no cost to the State. This choice to terminate will not prejudice your performance and payment securities or your rights to participate in future bidding for the project. If you choose to terminate the Contract, you must submit notification of the termination by USPS certified mail with a return receipt and certificate of mailing to the Office Engineer.

The notification must be postmarked before the effective date of the 2013-2014 FY Budget Act.

The Department notifies you when the 2013-2014 FY Budget Act becomes law.

DIVISION II GENERAL CONSTRUCTION 10 GENERAL

Add to section 10-1.02 of the RSS for section 10-1:

Notify Engineer not less than 7 working days before beginning work between PM 43.0 and 43.7 so public outreach efforts can occur.

12 TEMPORARY TRAFFIC CONTROL

Add to section 12-3.12C:

From PM 42.5 to 46.2 place portable changeable message signs at least 5 calendar days prior to beginning work to provide advance notification to the community. Message and placement will be as directed by the Engineer.

Start displaying the message on the portable changeable message sign 15 minutes before closing the lane.

Place the portable changeable message sign in advance of the 1st warning sign for each:

1. Stationary lane closure

Replace section 12-3.13 with:

12-3.13 IMPACT ATTENUATOR VEHICLE

12-3.13A General

12-3.13A(1) Summary

Section 12-3.13 includes specifications for protecting traffic and workers with an impact attenuator vehicle during moving lane closures.

Impact attenuator vehicles must comply with the following test levels under National Cooperative Highway Research Program 350:

- 1. Test level 3 if the preconstruction posted speed limit is 50 mph or more
- 2. Test levels 2 or 3 if the preconstruction posted speed limit is 45 mph or less

Comply with the attenuator manufacturer's instructions for:

- 1. Support truck
- 2. Trailer-mounted operation
- 3. Truck-mounted operation

Flashing arrow signs must comply with section 12-3.03. You may use a portable changeable message sign instead of a flashing arrow sign. If a portable changeable message sign is used as a flashing arrow sign, it must comply with section 6F.56 "Arrow Panels" of the *California MUTCD*.

12-3.13A(2) Definitions

impact attenuator vehicle: A support truck that is towing a deployed attenuator mounted to a trailer or a support truck with a deployed attenuator that is mounted to the support truck.

12-3.13A(3) Submittals

Upon request, submit a certificate of compliance for each attenuator used on the project.

12-3.13A(4) Quality Control and Assurance

Do not start impact attenuator vehicle activities until authorized.

Before starting impact attenuator vehicle activities, conduct a preinstallation meeting with the Engineer, subcontractors, and other parties involved with traffic control to discuss the operation of the impact attenuator vehicle during moving lane closures and when placing and removing components of stationary traffic control systems.

Schedule the location, time, and date for the preinstallation meeting with all participants. Furnish the facility for the preinstallation meeting within 5 miles of the job site or at another location if authorized.

12-3.13B Materials

Attenuators must be a brand on the Authorized Material List for highway safety features.

The combined weight of the support truck and the attenuator must be at least 19,800 pounds, except the weight of the support truck must not be less than 16,100 or greater than 26,400 pounds.

For the Trinity MPS-350 truck—mounted attenuator, the support truck must not have a fuel tank mounted underneath within 10'-6" of the rear of the support truck.

Each impact attenuator vehicle must have:

- 1. Legal brake lights, taillights, sidelights, and turn signals
- Inverted "V" chevron pattern placed across the entire rear of the attenuator composed of alternating 4-inch wide nonreflective black stripes and 4-inch wide yellow retroreflective stripes sloping at 45 degrees
- 3. Type II flashing arrow sign
- 4. Flashing or rotating amber light
- 5. Operable 2-way communication system for maintaining contact with workers

12-3.13C Construction

Except where prohibited, use an impact attenuator vehicle:

1. As a shadow vehicle in a moving lane closure.

Secure objects, including equipment, tools, and ballast on impact attenuator vehicles to prevent loosening upon impact by an errant vehicle.

Do not use a damaged attenuator in the work. Replace any attenuator damaged from an impact during work activities at your expense.

12-3.13 Payment

Not Used

Add to section 12-4.01:

Traffic delay is defined as the difference between the time it takes a vehicle to travel through the project at the posted speed limit when no work is in progress and the time it takes a vehicle to travel through the project when work is in progress.

Add to section 12-4.02A:

Designated holidays are as shown in the following table:

Designated Holidays

Holiday	Date observed							
New Year's Day	January 1st							
Washington's Birthday	3rd Monday in February							
Memorial Day	Last Monday in May							
Independence Day	July 4th							
Labor Day	1st Monday in September							
Veterans Day	November 11th							
Thanksgiving Day	4th Thursday in November							
Christmas Day	December 25th							

If a designated holiday falls on a Sunday, the following Monday is a designated holiday. If November 11th falls on a Saturday, the preceding Friday is a designated holiday.

Special days are:

Event	Date(s)
Plumas County Picnic	1st Saturday in June
Farmers Market	Every Thursday night June through September
High Sierra Music Festival	1st Thursday through Sunday in July
Plumas-Sierra County Fair	2nd Wednesday through Sunday in August
Fair Parade	1st Saturday in August
Hot August Nights	1st week in August
Mountain Harvest Festival	2nd Saturday in October

Special day dates are to be confirmed by the Engineer.

The full width of Route 70 roadway between PM 43.0 and 43.7 must be made available for the Fair Parade and be swept clean of loose debris and must provide a surface with no vertical drop-offs.

Under a 1-way reversing traffic control operation, traffic may be stopped in 1 direction for periods not to exceed 11 minutes. After each stoppage, all accumulated traffic for that direction must pass through the work zone before another stoppage is made. The Contractor must conduct operations so that the traffic delay does not exceed 15 minutes.

The maximum length of a stationary lane closure is:

Limits	Maximum Closure Length
PM 37.50 to 42.98	2.0 miles
PM 42.98 to 43.80	1.0 mile
PM 43.80 to 46.20	2.0 miles

Not more than 1 stationary lane closures will be allowed at one time.

Local authorities must be notified at least 7 calendar days before work begins that will close parking spaces or close a local road connection. You must cooperate with local authorities to handle traffic through the work area and must make arrangements to keep the work area clear of parked vehicles.

When a road connection or driveway is within the limits of traffic control, but not subject to active work, delays to motorists on the local road or driveway shall not be more than 15 minutes.

When a road connection or driveway is subject to active construction, it may be closed:

- 1. No more than one 30 minute period during micro-surfacing operations
- 2. No more than one 20 minute period during paving operations

Personal vehicles of your employees must not be parked on the traveled way or shoulders, including sections closed to traffic.

If work vehicles or equipment are parked within 6 feet of a traffic lane, close the shoulder area with fluorescent orange traffic cones or portable delineators. Place the cones or delineators on a taper in advance of the parked vehicles or equipment and along the edge of the pavement at 25-foot intervals to a point not less than 25 feet past the last vehicle or piece of equipment. Use at least 9 cones or delineators for the taper. Use a W20-1, "Road Work Ahead," W21-5b, "Right/Left Shoulder Closed Ahead," or C24(CA), "Shoulder Work Ahead," sign mounted on a crashworthy, portable sign support with flags. The sign must be placed as ordered by the Engineer and at least 48 by 48 inches in size. If a cone or delineator is displaced or overturned, immediately restore the device to its original position or location.

For multilane roadways, a minimum of 1 paved traffic lane not less than 12 feet wide must be open for use by traffic in each direction of travel.

For two-lane roadways, a minimum of 1 paved traffic lane not less than 12 feet wide must be open for use by traffic.

Except as allowed in Chart 2, the full width of the traveled way must be open to traffic on Fridays after 3:00 p.m., Saturdays, Sundays, Designated Holidays, Special Days, and when construction operations are not actively in progress.

Do not make lane closures if the atmospheric visibility is less than 1,000 feet.

Replace "Reserved" in section 12-4.04 with:

	Lano	Closuro		ion for E		ad Halia				
Thu	Fri	Sat	Sun	Mon	Tues	Wed	Thu	Fri	Sat	Sun
	Н									
х	XX	XX	xx							
	SD									
	XX									
		Н								
Х	XX	XX	XX							
		SD								
		XX								
			Н							
	Х	XX	XX	XX					1	
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Legen	d:									
		lane red	quiremer	nt charts						
Х	The full	width of	the trave	eled way	must be	open for	use by	traffic af	ter 3:00 p	o.m.
XX				eled way					•	
Н		ated holid		•						
SD	Special		-							

Replace "Reserved" in section 12-4.05F with:

	Chart no. 1																							
Conventional Highway Lane Requirements																								
County: Plumas	Route/Direction: 70 (EB & WB) PM: 42.98 to 43.80																							
Closure limits:																								
From hour to hour	24	1	2	3	4	5	6	7	8	9 1	01	11	2 1	31	41	51	61	71	81	192	021	22	23	24
Mondays through Thursdays								1	1	1	1	1	1	1	1	1	1	1						
Fridays								1	1	1	1	1	1	1	1	1								
Saturdays																								
Sundays																								
Legend: 1 Provide at least 1 through traffic lane open in direction of travel Work allowed within the highway where shoulder or lane closure is not required													_											
REMARKS: Use this chart for cold planning of	pera	atio	ons	6.																				

Replace "Reserved" in section 12-4.05F with:

<u> </u>					Cl	nar	t n	0.	2															
Con	Conventional Highway Lane Requirements																							
County: Plumas	R	out	te/[Dire	ecti	on:	7	0 (I	ΞВ	&	WI	3)	F	PM:	42	2.9	8 to	5 4	3.8	0				
Closure limits:																								
From hour to hour	24	1	2	3	4 :	5 (6	7	8 9	9	10	11:	12	131	4 1	51	61	7 1	81	92	02	12	22	3 24
Mondays through Thursdays	1	1	1	1	1	1														1	1	1	1	1
Fridays	1	1	1	1	1	1																		
Saturdays																								
Sundays																			1	1	1	1	1	1
Legend: 1 Provide at least 1 through Work allowed within the high														e is	no	t re	qui	ired	d					
REMARKS: Use this chart for HMA paving op	era	tioı	ns.																					

Replace section 12-5 with: 12-5 TRAFFIC CONTROL SYSTEM FOR LANE CLOSURE

12-5.01 GENERAL

Section 12-5 includes specifications for closing traffic lanes with stationary and moving lane closures on 2-lane, 2-way highways. The traffic control system for a lane closure must comply with the details shown.

Traffic control system includes signs.

12-5.02 MATERIALS

Vehicles equipped with attenuators must comply with section 12-3.13 of the special provisions.

A new attenuator that is proposed as equal to the authorized attenuators or attenuators ordered for recertification must not be used until authorized by METS.

12-5.03 CONSTRUCTION

12-5.03A General

During traffic striping, control traffic with a stationary or a moving lane closure. During other activities, control traffic with stationary lane closures.

Whenever components of the traffic control system are displaced or cease to operate or function as specified from any cause, immediately repair the components to the original condition or replace the components and restore the components to the original location.

12-5.03B Stationary Lane Closures

For a stationary lane closure made only for the work period, remove components of the traffic control system from the traveled way and shoulder, except for portable delineators placed along open trenches or excavation adjacent to the traveled way at the end of each work period. You may store the components at selected central locations designated by the Engineer within the limits of the highway.

For multilane highway lane closures, each vehicle used to place, maintain, and remove components of a traffic control system on a multilane highway must be equipped with a Type II flashing arrow sign that must be in operation whenever the vehicle is being used for placing, maintaining, or removing the components. Vehicles equipped with a Type II flashing arrow sign not involved in placing, maintaining or removing the components if operated within a stationary-type lane closure must only display the caution display mode. The sign must be controllable by the operator of the vehicle while the vehicle is in motion. If a flashing arrow sign is required for a lane closure, the flashing arrow sign must be operational before the lane closure is in place.

Additional flaggers are required at road connections and driveways when they are subject to active construction.

During reversing, one-way traffic control, use a pilot car to control traffic. The cones shown along the centerline need not be placed. The pilot car must have radio contact with personnel in the work area. Operate the pilot car through the traffic control zone at a speed not greater than 25 miles per hour.

12-5.03C Moving Lane Closures

A changeable message sign used in a moving lane closure must comply with section 12-3.12 except the sign must be truck-mounted. The full operational height to the bottom of the sign may be less than 7 feet above the ground but must be as high as practicable.

A flashing arrow sign used in a moving lane closure must be truck-mounted. Operate the flashing arrow sign in the caution display mode whenever it is being used on a 2-lane, 2-way highway.

12-5.04 PAYMENT

Traffic control system for lane closure is paid for as traffic control system. Flagging costs are paid for as specified in section 12-1.03.

The requirements in section 4-1.05 for payment adjustment do not apply to traffic control system. Adjustments in compensation for traffic control system will be made for an increase or decrease in traffic control work if ordered and will be made on the basis of the cost of the necessary increased or decreased traffic control. The adjustment will be made on a force account basis for increased work and estimated on the same basis in the case of decreased work.

A traffic control system required by change order work is paid for as a part of the change order work.

Replace section 12-8 with: 12-8 TEMPORARY PAVEMENT DELINEATION

12-8.01 GENERAL

Section 12-8 includes specifications for placing, applying, maintaining, and removing temporary pavement delineation.

Painted traffic stripe used for temporary delineation must comply with section 84-3. Apply 1 or 2 coats.

Temporary signing for no-passing zones must comply with section 12-3.06.

12-8.02 MATERIALS

12-8.02A General

For micro-surfacing applications, temporary raised pavement markers must be one of the temporary pavement markers on the Authorized Material List for short-term day/night use, 14 days or less.

Place temporary raised pavement markers under the manufacturer's instructions.

12-8.02B Temporary Lane Line and Centerline Delineation

Temporary pavement markers must be the same color as the lane line or centerline markers being replaced. Temporary pavement markers must be one of the temporary pavement markers on the Authorized Material List for short-term day or night use, 14 days or less, or long-term day or night use, 180 days or less.

12-8.02C Temporary Edge Line Delineation

Temporary, removable, construction-grade striping and pavement marking tape must be one of the types on the Authorized Material List. Apply temporary, removable, construction-grade striping and pavement marking tape under the manufacturer's instructions.

12-8.03 CONSTRUCTION

12-8.03A General

Whenever work activities obliterate pavement delineation, place temporary or permanent pavement delineation before opening the traveled way to traffic. Place lane line and centerline pavement delineation for traveled ways open to traffic. On multilane roadways, freeways, and expressways, place edge line delineation for traveled ways open to traffic.

Before applying micro-surfacing that will obliterate existing traffic stripes, place temporary raised pavement markers on the existing traffic stripes except right edge lines at intervals of not more than 24 feet. Place 2 markers side by side on double traffic stripes, 1 on each stripe longitudinally at intervals not exceeding 24 feet. Before opening the lanes to uncontrolled traffic, remove the covers from the temporary raised pavement markers.

Establish the alignment for temporary pavement delineation, including required lines or markers. Surfaces to receive an application of paint or removable traffic tape must be dry and free of dirt and loose material. Do not apply temporary pavement delineation over existing pavement delineation or other temporary pavement delineation. Maintain temporary pavement delineation until it is superseded or you replace it with a new striping detail of temporary pavement delineation or permanent pavement delineation.

Place temporary pavement delineation on or adjacent to lanes open to traffic for a maximum of 14 days. Before the end of the 14 days, place the permanent pavement delineation. If the permanent pavement delineation is not placed within the 14 days, replace the temporary pavement markers with additional temporary pavement delineation equivalent to the striping detail specified for the permanent pavement delineation for the area. The Department does not pay for the additional temporary pavement delineation.

When the Engineer determines the temporary pavement delineation is no longer required for the direction of traffic, remove the markers, underlying adhesive, and removable traffic tape from the final layer of surfacing and from the existing pavement to remain in place. Remove temporary pavement delineation that conflicts with any subsequent or new traffic pattern for the area.

12-8.03B Temporary Lane Line and Centerline Delineation

Whenever lane lines or centerlines are obliterated, the minimum lane line and centerline delineation must consist of temporary pavement markers placed longitudinally at intervals not exceeding 24 feet. The temporary pavement markers must be temporary pavement markers on the Authorized Material List for short-term day or night use, 14 days or less, or long-term day or night use, 180 days or less. Place temporary pavement markers under the manufacturer's instructions. Cement the markers to the surfacing with the adhesive recommended by the manufacturer, except do not use epoxy adhesive to place pavement markers in areas where removal of the markers will be required.

For temporary lane line or centerline delineation consisting entirely of temporary pavement markers, place the markers longitudinally at intervals not exceeding 24 feet.

Where no-passing centerline pavement delineation is obliterated, install the following temporary no-passing zone signs before opening lanes to traffic. Install a W20-1, "Road Work Ahead," sign from 1,000 feet to 2,000 feet in advance of a no-passing zone. Install a R4-1, "Do Not Pass," sign at the beginning of a no-passing zone and at 2,000-foot intervals within the no-passing zone. For continuous zones longer than 2 miles, install a W7-3a or W71(CA), "Next ____ Miles," sign beneath the W20-1 sign. Install a R4-2, "Pass With Care," sign at the end of the no-passing zone. The Engineer determines the exact location of temporary no-passing zone signs. Maintain the temporary no-passing zone signs in place until you place the permanent no-passing centerline pavement delineation. Remove the temporary no-passing zone signs when the Engineer determines they are no longer required for the direction of traffic.

12-8.03C Temporary Edge Line Delineation

Whenever edge lines are obliterated on multilane roadways, freeways, and expressways, place edge line delineation for that area adjacent to lanes open to traffic consisting of (1) solid, 4-inch wide traffic stripe tape of the same color as the stripe being replaced, (2) traffic cones, (3) portable delineators or channelizers placed longitudinally at intervals not exceeding 100 feet. You may apply temporary painted traffic stripe where removal of the 4-inch wide traffic stripe will not be required.

The Engineer determines the lateral offset for traffic cones, portable delineators, and channelizers used for temporary edge line delineation. If traffic cones or portable delineators are used for temporary pavement delineation for edge lines, maintain the cones or delineators during hours of the day when the cones or delineators are being used for temporary edge line delineation.

Channelizers used for temporary edge line delineation must be an orange surface-mounted type. Cement channelizer bases to the pavement as specified in section 85 for cementing pavement markers to pavement except do not use epoxy adhesive to place channelizers on the top layer of the pavement. Channelizers must be one of the 36-inch, surface-mounted types on the Authorized Material List.

Remove the temporary edge line delineation when the Engineer determines it is no longer required for the direction of traffic.

12-8.04 PAYMENT

Not Used

14 ENVIRONMENTAL STEWARDSHIP

Replace section 14-11.07 with:

14-11.07 REMOVE YELLOW TRAFFIC STRIPE AND PAVEMENT MARKING WITH HAZARDOUS WASTE RESIDUE

14-11.07A General

14-11.07A(1) Summary

Section 14-11.07 includes specifications for removing existing yellow thermoplastic and yellow painted traffic stripe and pavement marking. The residue from the removal of this material is a Department-generated hazardous waste.

Residue from removal of yellow thermoplastic and yellow painted traffic stripe and pavement marking contains lead chromate. The average lead concentration is at least 1,000 mg/kg total lead or 5 mg/l soluble lead. When applied to the roadway, the yellow thermoplastic and yellow painted traffic stripe and pavement marking contained as much as 2.6 percent lead. Residue produced from the removal of this yellow thermoplastic and yellow painted traffic stripe and pavement marking contains heavy metals in concentrations that exceed thresholds established by the Health & Safety Code and 22 CA Code of Regs. For bidding purposes, assume the residue is not regulated under the Federal Resource Conservation and Recovery Act (RCRA), 42 USC § 6901 et seq.

Yellow thermoplastic and yellow paint may produce toxic fumes when heated.

14-11.07A(2) Submittals 14-11.07A(2)(a) General

Reserved

14-11.07A(2)(b) Lead Compliance Plan

Submit a lead compliance plan under section 7-1.02K(6)(j)(ii).

14-11.07A(2)(c) Work Plan

Submit a work plan for the removal, containment, storage, and disposal of yellow thermoplastic and yellow painted traffic stripe and pavement marking. The work plan must include:

- 1. Objective of the operation
- 2. Removal equipment
- 3. Procedures for removal and collection of yellow thermoplastic and yellow painted traffic stripe and pavement marking residue, including dust
- 4. Type of hazardous waste storage containers
- 5. Container storage location and how it will be secured
- 6. Hazardous waste sampling protocol and QA/QC requirements and procedures
- 7. Qualifications of sampling personnel
- 8. Analytical lab that will perform the analyses
- DTSC registration certificate and CA Highway Patrol (CHP) Biennial Inspection of Terminals (BIT)
 Program compliance documentation of the hazardous waste hauler that will transport the hazardous waste
- 10. Disposal site that will accept the hazardous waste residue

The Engineer will review the work plan within 5 business days of receipt.

Do not perform work that generates hazardous waste residue until the work plan has been authorized.

Correct any rejected work plan and resubmit a corrected work plan within 5 business days of notification by the Engineer. A new review period of 5 business days will begin from date of resubmittal.

14-11.07A(2)(d) Analytical Test Results

Submit analytical test results of the residue from removal of yellow thermoplastic and yellow painted traffic stripe and pavement marking, including chain of custody documentation, for review and acceptance before:

- 1. Requesting the Engineer's signature on the waste profile requested by the disposal facility
- 2. Requesting the Engineer obtain an US EPA Generator Identification Number for disposal
- 3. Removing the residue from the site

14-11.07A(2)(e) U.S. Environmental Protection Agency Identification Number Request

Submit a request for the US EPA Generator Identification Number when the Engineer accepts analytical test results documenting that residue from removal of yellow thermoplastic and yellow painted traffic stripe and pavement marking is a hazardous waste.

14-11.07A(2)(f) Disposal Documentation

Submit documentation of proper disposal from the receiving landfill within 5 business days of residue transport from the project.

14-11.07B Materials

Not Used

14-11.07C Construction

Where grinding or other authorized methods are used to remove yellow thermoplastic and yellow painted traffic stripe and pavement marking that will produce a hazardous waste residue, immediately contain and collect the removed residue, including dust. Use a HEPA filter-equipped vacuum attachment operated concurrently with the removal operations or other equally effective approved methods for collection of the residue.

Make necessary arrangements to test the yellow thermoplastic and yellow paint hazardous waste residue as required by the disposal facility and these special provisions. Testing must include:

- 1. Total lead by US EPA Method 6010B
- 2. Total chromium by US EPA Method 6010B
- 3. Soluble lead by California Waste Extraction Test (CA WET)
- 4. Soluble chromium by CA WET
- 5. Soluble lead by Toxicity Characteristic Leaching Procedure (TCLP)
- 6. Soluble chromium by TCLP

From the first 220 gal of hazardous waste or portion thereof if less than 220 gal of hazardous waste are produced, a minimum of 4 randomly selected samples must be taken and analyzed individually. Samples must not be composited. From each additional 880 gal of hazardous waste or portion thereof if less than 880 gal are produced, a minimum of 1 additional random sample must be taken and analyzed. Use chain of custody procedures consistent with chapter 9 of US EPA Test Methods for Evaluating Solid Waste, Physical/Chemical Methods (SW-846) while transporting samples from the project to the laboratory. Each sample must be homogenized before analysis by the laboratory performing the analyses. A sample aliquot sufficient to cover the amount necessary for the total and the soluble analyses must then be taken. This aliquot must be homogenized a 2nd time and the total and soluble analyses run on this aliquot. The homogenization process must not include grinding of the samples. Submit the name and location of the disposal facility that will be accepting the hazardous waste and the analytical laboratory along with the testing requirements not less than 5 business days before the start of removal of yellow thermoplastic and yellow painted traffic stripe and pavement marking. The analytical laboratory must be certified by the California Department of Public Health (CDPH) Environmental Laboratory Accreditation Program (ELAP) for all analyses to be performed.

After the Engineer accepts the analytical test results, dispose of yellow thermoplastic and yellow paint hazardous waste residue at a Class 1 disposal facility located in California under the requirements of the disposal facility operator within 90 days after accumulating 220 pounds of residue and dust.

If less than 220 pounds of hazardous waste residue and dust is generated in total, dispose of it within 90 days after the start of accumulation of the residue and dust.

The Engineer will sign all manifests as the generator within 2 business days of receiving and accepting the analytical test results and receiving your request for the US EPA Generator Identification Number. Use a transporter with a current DTSC registration certificate and that is in compliance with the CHP BIT Program when transporting hazardous waste.

14-11.07D Payment

Payment for a lead compliance plan is not included in the payment for environmental stewardship work.

If analytical test results demonstrate that the residue is a non-hazardous waste and the Engineer agrees, dispose of the residue at an appropriately permitted CA Class II or CA Class III facility. The Department does not adjust payment for this disposal.

Replace section 14-11.09 with:

14-11.09 TREATED WOOD WASTE

14-11.09A General

14-11.09A(1) Summary

Section 14-11.09 includes specifications for handling, storing, transporting, and disposing of treated wood waste (TWW).

Wood removed from reconstruct metal beam guard railing is TWW. Manage TWW under 22 CA Code of Regs, Div. 4.5, Chp. 34.

14-11.09A(2) Submittals

For disposal of TWW, submit as an informational submittal a copy of each completed shipping record and weight receipt within 5 business days.

14-11.09B Materials

Not Used

14-11.09C Construction

14-11.09C(1) General

14-11.09C(2) Training

Provide training to personnel who handle TWW or may come in contact with TWW. Training must include:

- 1. All applicable requirements of 8 CA Code of Regs
- 2. Procedures for identifying and segregating TWW
- 3. Safe handling practices
- 4. Requirements of 22 CA Code of Regs, Div. 4.5, Chp. 34
- 5. Proper disposal methods

Maintain records of personnel training for 3 years.

14-11.09C(3) Storage

Store TWW before disposal using the following methods:

- 1. Elevate on blocks above a foreseeable run-on elevation and protect from precipitation for no more than 90 days.
- Place on a containment surface or pad protected from run-on and precipitation for no more than 180 days.
- 3. Place in water-resistant containers designed for shipping or solid waste collection for no more than 1 year.
- 4. Place in a storage building as defined in 22 CA Code of Regs, Div. 4.5, Chp. 34, § 67386.6(a)(2)(C).

Prevent unauthorized access to TWW using a secured enclosure such as a locked chain link fenced area or a lockable shipping container located within the job site.

Resize and segregate TWW at a location where debris from the operation including sawdust and chips can be contained. Collect and manage the debris as TWW.

Provide water-resistant labels that comply with 22 CA Code of Regs, Div. 4.5, Chp. 34, §67386.5, to clearly mark and identify TWW and accumulation areas. Labels must include:

- 1. Caltrans, District number, Construction, Construction Contract number
- 2. District office address
- 3. Engineer's name, address, and telephone number
- 4. Contractor's contact name, address and telephone number
- 5. Date placed in storage

14-11.09C(4) Transporting and Disposal

Before transporting TWW, obtain an agreement from the receiving facility that the TWW will be accepted. Protect shipments of TWW from loss and exposure to precipitation. For projects with 10,000 pounds or more of TWW, request a US EPA Generator Identification Number from the Engineer at least 5 business days before the first shipment. Each shipment must be accompanied by a shipping record such as a bill of lading or invoice that includes:

- 1. Caltrans with district number
- 2. Construction Contract number
- 3. District office address
- 4. Engineer's name, address, and telephone number
- 5. Contractor's contact name and telephone number
- 6. Receiving facility name and address
- 7. Waste description: Treated Wood Waste with preservative type if known or unknown/mixture
- 8. Project location
- 9. Estimated quantity of shipment by weight or volume
- 10. Date of transport
- 11. Date of receipt by the receiving TWW facility
- 12. Weight of shipment as measured by the receiving TWW facility
- 13. For projects with 10,000 pounds or more of TWW include the USA EPA Generator Identification Number.

The shipping record must be at least a 4-part carbon or carbonless 8 1/2 by 11-inch form to allow retention of copies by the Engineer, transporter, and disposal facility.

Dispose of TWW at an approved TWW facility. A list of currently approved TWW facilities is available at:

http://www.dtsc.ca.gov/HazardousWaste/upload/lanfillapr11pdated1.pdf

Dispose of TWW within:

- 1. 90 days of generation if stored on blocks
- 2. 180 days of generation if stored on a containment surface or pad
- 3. 1 year of generation if stored in a water-resistant container, or within 90 days after the container is full, whichever is shorter
- 4. 1 year of generation if storing in a storage building as defined in 22 CA Code of Regs, Div. 4.5, Chp. 34, § 67386.6(a)(2)(C)

14-11.09D Payment

Not Used

15 EXISTING FACILITIES

Replace section 15-1.03C with:

15-1.03C Loop Detectors

The existing inductive loop detectors shown must remain in place and operational.

If you damage part of a loop conductor including an adjacent loop and the portion leading to the adjacent pull box, replace the entire loop detector within 2 weeks of damage.

Replace section 15-2.02B(3) with:

15-2.02B(3) Cold Planing Asphalt Concrete Pavement

15-2.02B(3)(a) General

Schedule cold planing activities so that not more than 4 days elapses between the time the pavement is cold planed and the micro-surfacing or HMA is placed.

15-2.02B(3)(b) Materials

Use the same quality of HMA for temporary tapers that is used for the HMA overlay or comply with the specifications for minor HMA in section 39.

15-2.02B(3)(c) Construction

15-2.02B(3)(c)(i) General

Do not use a heating device to soften the pavement.

The cold planing machine must be:

- 1. Equipped with a cutter head width that matches the planing width. If the cutter head width is wider than the cold plane area shown, submit to the Engineer a request for using a wider cutter head. Do not cold plane unless the Engineer approves your request.
- 2. Equipped with automatic controls for the longitudinal grade and transverse slope of the cutter head and:
 - 2.1. If a ski device is used, it must be at least 30 feet long, rigid, and a 1-piece unit. The entire length must be used in activating the sensor.
 - 2.2. If referencing from existing pavement, the cold planing machine must be controlled by a self-contained grade reference system. The system must be used at or near the centerline of the roadway. On the adjacent pass with the cold planing machine, a joint-matching shoe may be used.
- 3. Equipped to effectively control dust generated by the planing operation
- 4. Operated so that no fumes or smoke is produced.

Replace broken, missing, or worn machine teeth.

15-2.02B(3)(c)(ii) Grade Control and Surface Smoothness

Furnish, install, and maintain grade and transverse slope references.

The depth, length, width, and shape of the cut must be as shown or as ordered. The final cut must result in a neat and uniform surface. Do not damage the remaining surface.

The completed surface of the planed asphalt concrete pavement must not vary more than 0.02 foot when measured with a 12-foot straightedge parallel with the centerline. With the straightedge at right angles to the centerline, the transverse slope of the planed surface must not vary more than 0.03 foot.

Where lanes are open to traffic, the drop-off of between adjacent lanes must not be more than 0.15 foot.

15-2.02B(3)(c)(iii) Temporary HMA Tapers

Not Used

15-2.02B(3)(c)(iv) Remove Planed Material

Remove cold planed material concurrent with planing activities so that removal does not lag more than 50 feet behind the planer.

15-2.02B(3)(d) Payment

Payment for removal of pavement markers, thermoplastic traffic stripe, painted traffic stripe, and pavement marking within the area of cold planing is included in the payment for cold plane asphalt concrete pavement of the types shown in the Bid Item List.

Replace section 15-2.02C(2) with:

15-2.02C(2) Remove Traffic Stripes and Pavement Markings Containing Lead

Residue from removing traffic stripes and pavement markings contains lead from the paint or thermoplastic. The average lead concentrations are less than 1,000 mg/kg total lead and 5 mg/L soluble lead. This residue:

- 1. Is a nonhazardous waste
- Does not contain heavy metals in concentrations that exceed thresholds established by the Health and Safety Code and 22 CA Code of Regs
- 3. Is not regulated under the Federal Resource Conservation and Recovery Act (RCRA), 42 USC § 6901 et seq.

Submit a lead compliance plan under section 7-1.02K(6)(j)(ii).

Payment for a lead compliance plan is not included in the payment for existing facilities work.

Payment for handling, removal, and disposal of pavement residue that is a nonhazardous waste is included in the payment for the type of removal work involved.

Replace "Reserved" in section 15-2.04D with:

15-2.04D(1) General

Cable anchor assemblies or terminal anchor assemblies, including concrete anchors and steel foundation tubes that are not reused must be completely removed.

Reconstruction includes single or double metal beam guard railing and thrie beam barrier, transition railings, end anchor assemblies and terminal systems where shown.

Posts, blocks, and hardware must comply with section 83-1.02B.

15-2.04D(2) Materials

Reconstruct metal beam guard railing with steel posts.

Terminal systems must use the type of post and block specified by the Manufacturer and as specified in 83-1.02C or as shown.

End anchors and transition railings must use the type of post and block shown.

Sand bedding must comply with section 19-3.02E(2).

15-2.04D(3) Construction

Reconstruct metal beam guard railing with new posts, blocks and hardware.

Acceptable salvaged steel tubes, plates, brackets, and cables may be reused. Keep a sufficient supply of new or salvaged spare parts on hand to insure there is no delay to the work.

Reconstruct terminal system in accordance with Manufacturer's instructions and as specified in 83-1.02C or as shown. Each terminal system installed must be identified by painting the type of terminal system in neat black letters and figures 2 inches high on the backside of the rail element between system posts numbers 4 and 5.

Sand bedding or other acceptable material must be used to:

- 1. Backfill holes resulting from the removal of existing posts and anchors that are not reused
- 2. Partially backfill holes so that the bottom of the post bears on compacted material if a post of the same size is being replaced
- Completely backfill holes resulting from the removal of a wood post before installing a steel post in the same hole
- 4. Completely backfill holes resulting from drilling a pilot hole larger than 5 inches before driving a steel post in the hole
- 5. Backfill any void remaining after installing a post or sleeve

Compaction must comply with section 19-5. Place material in lifts approximately 4 inch thick. Moisten and thoroughly compact each layer.

Identify 7' steel posts by painting a 2-inch tall numeral 7 in the web near the top before installation. Use black metallic acrylic resin type paint applied to clean dry surface.

15-2.04D(4) Payment

Reconstruct metal beam guard railing will be measured along the face of the rail from end to end regardless of the type of rail, terminal system, or anchor that is reconstructed.

Furnishing new replacement parts for unacceptable salvaged steel tubes, plates, brackets, or cables is change order work.

Replace paragraph 5 of section 15-2.10B with:

Instead of using new materials similar in character to those in the existing structure, you may use raising devices to adjust a manhole to grade. Before starting paving work, measure, and fabricate, raising devices. Raising devices must:

- 1. Comply with the specifications for section 75 except that galvanizing is not required
- 2 Have a shape and size that matches the existing frame
- 3. Be match marked by painting identification numbers on the device and corresponding structure
- 4. Result in an installation that is equal to or better than the existing one in stability, support, and nonrocking characteristics
- 5. Be fastened securely to the existing frame without projections above the surface of the road or into the clear opening

Replace section 15-2.10D with:

15-2.10D Adjust Metal Beam Guard Railing

15-2.10D(1) General

Adjust metal beam guard railing to the height shown.

15-2.10D(2) Materials

Sand must comply with section 19-3.02E(2).

15-2.10D(3) Construction

Adjust guard railing by either:

- 1. Removing and reinstalling the post with a sufficient quantity of sand in the hole so that when the post is reinstalled at the proper height it rests on firmly compacted material.
- 2. Raising the element and block to proper height and drilling a new hole or use an existing hole in the post if the block will not be more than 2 inches above the top of the post. Offset the new hole approximately 1 inch towards the leading edge of the post (towards approaching traffic) as measured from the vertical plane of the original hole.

Use only one of the above methods to adjust guard rail in any individual run unless authorized.

Furnish replacements for unacceptable posts, blocks, or hardware. A sufficient supply of new posts, blocks and hardware must be on hand to insure there is no delay to the work.

15-2.10D(4) Payment

Adjust metal beam guard railing is measured from end to end along the face of the adjusted railing including along and around the face of the return, end, and terminal sections.

Furnishing replacements for unacceptable posts, blocks, or hardware will be paid by force account.

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DIVISION V SURFACINGS AND PAVEMENTS 37 BITUMINOUS SEALS

Add to section 37-3.02A:

Aggregate for micro-surfacing must be Type III.

Add to section 37-3.01C(1):

At least 10 days before Just-In-Time-Training (JITT), submit:

- 1. The instructor's name and listed experience
- 2. A copy of the course syllabus, handouts, and presentation materials
- 3. The JITT facility's location
- 4. A list of your staff, and their assignments, who will be attending the JITT

Add to section 37-3.01D(1):

37-3.01D(1A) Just-In-Time Training

JITT is a formal training class for Micro-Surfacing and Micro-Surfacing techniques. JITT must be:

- 1. At least 2 hours long
- 2. At your option, an extension of the prepaving conference
- 3. Completed within 5 business days before you begin Micro-Surfacing work
- 4. Conducted during normal working hours

The following staff must complete Just-In-Time-Training (JITT):

- 1. Project manager
- 2. Project superintendent
- 3. Quality control staff
- 4. Equipment Operators

Provide a JITT instructor who is experienced with Micro-Surfacing methods, materials, and tests. The instructor must not be your employee, a material supplier, or a Department field staff member. Upon completion of JITT, the instructor must issue a certificate of completion to the participants.

Personnel who have completed equivalent JITT training within the preceding 12 months must submit certificates of completion when requesting exemption from attending this JITT.

Just-In-Time Training participation does not relieve you of responsibility under the contract for the successful completion of the work in conformance with the requirements of the plans and specifications.

Replace the row for Residue by evaporation, min in the table in the 3rd paragraph of section 37-3.02C(3) with:

Residue by evaporation, min	California Test 331	64%

Replace the 1st paragraph of section 37-3.03C(5)(a) with:

Use a continuous self-loading mixing machine except you may use truck mounted mixer spreaders on any of the following:

- 1. Radii'
- 2. Side streets
- Gore areas
- 4. Areas requiring hand work

Replace "Reserved" in section 37-3.03D(3)(c) with:

Construct a micro-surfacing test strip using a Continuous Self-Loading Mixing Machine:

- 1. From 1000 to 1200 feet long
- 2. The same as the full production micro-surfacing
- 3. On one of the application courses specified at an authorized location
- 4. At the same time of day or night the full production micro-surfacing is to be applied

If multiple application courses are specified, you may construct test strips over 2 days or nights.

The Engineer evaluates the test strip during application for compliance with section 37-3.03D(4)(c)(iii).

The Engineer evaluates the test strip after traffic has used it for 12 hours for compliance with section 37-3.03D(4)(c)(iv)

If the Engineer determines the mix design or placement procedure is unacceptable, make modifications and construct a new test strip for the Engineer's evaluation.

Add to section 37-3.03D(2)(a):

Before applying micro-surfacing, remove thermoplastic stripe, pavement markers and underlying adhesive. Must conform with Section 15-2.02D.

Replace "Reserved" in section 37-3.03D(3)(c) with:

Construct a micro-surfacing test strip:

- 1. From 300 to 450 feet long
- 2. The same as the full production micro-surfacing
- 3. On one of the application courses specified at an authorized location
- 4. At the same time of day or night the full production micro-surfacing is to be applied

If multiple application courses are specified, you may construct test strips over 2 days or nights.

The Engineer evaluates the test strip after traffic has used it for 12 hours. If the Engineer determines the mix design or placement procedure is unacceptable, make modifications and construct a new test strip for the Engineer's evaluation.

Replace table in the 1st paragraph of section 37-3.03D(4)(c)(i) with:

Micro-surfacing Spread Rates

Micro-surfacing type	Location	Range (lb of dry aggregate/sq yd)					
Type II	Full lane width	10–20					
Type III	Full lane width	30–32					

Add to section 37-3.04A:

If there is no bid item for remove pavement markers, removing pavement markers is included in applying the micro surfacing.

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39 HOT MIX ASPHALT

Add to section 39-1.01:

Produce and place HMA Type A under the Standard construction process.

Add to section 39-1.01A:

For HMA Type A, B, and HMA with warm mix asphalt technology do not pave on the traveled way between November 1 and May 1 if:

- 1. The quantity of HMA is greater than 1000 tons or
- 2. The project elevation is greater than 1500 feet

For HMA-O, RHMA-G, RHMA-O, or RHMA-O-HB do not pave on the traveled way between September 15 and May 1.

Replace the 1st paragraph of section 39-1.02B with:

Tack coat must comply with the specifications for asphaltic emulsion or asphalts. Use CRS2, CQS1, asphalt binder, or PMCRS2 asphaltic emulsion.

Add to section 39-1.02C:

Asphalt binder used in HMA Type A must be PG 64-28 M.

Add to section 39-1.02E:

Aggregate used in HMA Type A must comply with the 1/2-inch HMA Types A and B gradation.

If aggregate source is from Modoc, Siskiyou, or Shasta County, submit aggregate samples to the Engineer at least 30 days before the aggregate's intended use.

Treat HMA aggregate with lime using the slurry method.

Add to the 4th table of section 39-1.02E:

Aggregate Quality

Quality characteristic	naracteristic Test method HMA type								
		Α	В	RHMA-G	OGFC				
Sodium sulfate soundness (% max loss) ^c	California Test 214	25	25	25	25				
Coarse durability index (min) d	California Test 229	65	65	65	65				
Fine durability index (min)	California Test 229	50	50	50	50				
Plasticity Index	California Test 204	<10	<10	<10					

^c Requirement applies only if aggregate source is from Modoc, Siskiyou, or Shasta County.

Replace the 2nd row of the 4th table of section 39-1.02E with:

Aggregate Quality

Quality characteristic	Test method	HMA type			
		Α	В	RHMA-G	OGFC
Los Angeles Rattler (% max.)	California				
Loss at 100 rev.	Test 211	12		12	12
Loss at 500 rev.		25	25	25	25

Replace the 3rd paragraph of section 39-1.03A with:

Laboratories testing aggregate qualities, RAP, and preparing the mix design and JMF must be qualified under the Department's Independent Assurance Program. Take samples under California Test 125.

When doing your mix design take three 80 lb RAP samples from stockpiles under California Test 125. Split each sample into 2 parts:

- 1. Each part must weigh at least 40 lb.
- 2. Submit 1 part to the Engineer with the JMF.
- 3. Use 1 part for your testing.

Add to the 1st table of the RSS for section 39-1.03B:

HMA Mix Design Requirements

Quality characteristic	Test		HMA type		
	method				
		Α	В	RHMA-G	
Optimum Bitumen Content	California			7.0% min	
(OBC)	Test 367				
Tensile Strength Ratio b	California	≥80	≥80	Report only	
_	Test 371				

^b After lime treatment.

d Requirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta County.

 $^{^{\}rm c}$ Determine the following using AASHTO T84: bulk specific gravity (SSD) of fine aggregate, bulk specific gravity (oven dry) of fine aggregate, $G_{\rm r}$, and absorption of fine aggregate.

Replace the 1st row of the 1st table of the RSS for section 39-1.03B with:

HMA Mix Design Requirements

Quality characteristic	Test	HMA type		
	method	Α	В	RHMA-G
Air void content (%)	California Test 367°	4.0	4.0	3.5

Replace the 2nd row of the 1st table of the RSS for section 39-1.03B with:

HMA Mix Design Requirements

Quality characteristic	Test	HMA type		
	method	Α	В	RHMA-G
Voids in mineral aggregate (% min.)	California			
No. 4 grading	Test 367 ^c	17.0	17.0	
3/8" grading		15.0	15.0	
1/2" grading		14.0	14.0	18.0-23.0
3/4" grading		13.0	13.0	18.0–23.0

Add to section 39-1.03B:

If the project is greater than 1500 feet elevation, perform a mix design that produces the quality characteristic shown in the table when mixed with the asphalt used on the project in the amount determined to be optimum by California Test 367:

Quality Characteristic	Test	Requirement
Surface abrasion	California Test 360	Loss not to exceed 0.4 g/cm ²

Replace the 4th and 5th paragraphs of section 39-1.03C with:

For HMA Type A, B or RHMA-G submit test results with the JMF submittal for:

- 1. California Test 204 plasticity index
- 2. California Test 371 tensile strength ratio for treated and untreated HMA
- 3. Minimum dry strength, for RAP substitution greater than 15 percent
- 4. AASHTO T 324 (Modified), for RAP substitution greater than 15 percent

Replace the 6th paragraph of section 39-1.03C with:

For HMA Type A, B or RHMA-G submit the California Test 371 test results to:

- 1. The Engineer
- 2. Moisture_Tests@dot.ca.gov

Delete the last two paragraphs of the RSS for Section 39-1.03C.

Delete the last paragraph of section 39-1.03E.

Delete "If required," from the 5th item in the 2nd paragraph of the RSS for section 39-1.03G.

Replace the 3rd paragraph of the RSS for section 39-1.03G with:

With an accepted modified JMF submittal, the Engineer verifies each modified JMF within 10 business days of receiving all verification samples.

Replace the 4th paragraph of the RSS for section 39-1.03G with:

The Engineer verifies the modified JMF after the modified JMF HMA is placed on the project and verification samples are taken within the first 750 tons following sampling requirements in section 39-1.03E, "Job Mix Formula Verification." The Engineer tests verification samples for compliance with:

- 1. Stability as shown in the table titled "HMA Mix Design Requirements"
- 2. Air void content at
 - 2.1. design value ±2.0 percent for HMA Type A and Type B
 - 2.2. design value ±1.5 percent for RHMA-G
- 3. Voids in mineral aggregate as shown in the table titled "HMA Mix Design Requirements"
- 4. Voids filled with asphalt, report only
- 5. Dust proportion, report only
- 6. Optimum Bitumen Content
- 7. Tensile Strength Ratio

Replace the last paragraph of the RSS for section 39-1.03G with:

The Engineer deducts \$4,000 from payments for each modified JMF verification.

Add to section 39-1.08A:

On the first production day and once during production of the first 5,000 tons, submit:

- 1. Samples split from your HMA production sample for California Test 371 to:
 - 1.1 The Engineer
 - 1.2 The Transportation Laboratory, Attention: Moisture Test.
- 2. The California Test 371 results to:
 - 2.1 The Engineer
 - 2.2 Moisture_Tests@dot.ca.gov

After the 1st production day and production of the first 5,000 tons, submit the California Test 371 results for each 5,000 tons to:

- 1. The Engineer
- 2. Moisture_Tests@dot.ca.gov

Add to section 39-1.11A of the RSS for section 39-1.11:

Place RHMA-G only when the atmospheric temperature is 70 degrees F or greater.

Use a material transfer vehicle (MTV) if:

- 1. The project quantity of hot mix asphalt to be paved is greater than 1000 tons, and
- 2. Any of the following exists:
 - 2.1. Paving is allowed and the atmospheric temperature is below 70 degrees F.
 - 2.2. Time from discharge to truck at the HMA plant until transfer to the paver's hopper is 90 minutes or greater.

The MTV must:

- 1. Either receive HMA directly from the truck or use a pickup head to load it from a windrow than can be deposited on the roadway surface for a maximum of 100 feet in length.
- 2. Remix the HMA, with augers, before loading the paver.
- 3. Transfer HMA directly into the paver's receiving hopper or feed system.
- 4. Have sufficient capacity to prevent stopping the paver.

The MTV requirements will not apply to replace asphalt concrete surfacing under section 39-1.21.

Remove pavement markers prior to applying tack coat or placing HMA.

If there is no bid item for remove pavement markers, removing pavement markers is included in HMA (Type A).

Replace the 2nd, 3rd, and 4th paragraphs of section 39-1.11B(1) of the RSS for section 39-1.11 with:

Place HMA on adjacent traveled way lanes so that at the end of each work shift the distance between the ends of HMA layers on adjacent lanes is from 5 to 10 feet. Place additional HMA along the transverse edge at each lane's end and along the exposed longitudinal edges between adjacent lanes. Hand rake and compact the additional HMA to form temporary conforms. You may place Kraft paper or another authorized bond breaker under the conform tapers to facilitate the taper removal when paving operations resume.

Delete section 39-1.11B(2) of the RSS for section 39-1.11.

Replace the headings and paragraphs in section 39-1.12 including the RSS for section 39-12 with: 39-1.12A General

Section 39-1.12 includes specifications for measuring pavement smoothness with an inertial profiler (IP) and straightedge, analyzing the data with FHWA's engineering software ProVAL, and correcting deficient smoothness.

Test pavement smoothness using an IP except use a 12-foot straightedge at the following locations:

- 1. Traffic lanes less than 1,000 feet in length including ramps, turn lanes, and acceleration and deceleration lanes
- 2. HMA pavement within 3 feet from and parallel to the construction joint formed between curbs, gutters, or existing pavement
- 3. Areas within 15 feet of manholes
- 4. Shoulders
- 5. Weigh-in-motion areas
- 6. Miscellaneous areas such as medians, gore areas, turnouts, and maintenance pullouts

Where IP testing is required, pavement smoothness for each lane must be determined by the international roughness index (IRI) for the left and right wheel paths in an individual lane and then averaging the results. The average of the IRIs from the left and right wheel paths for the same lane is the mean roughness index (MRI) of the lane. The wheel paths are a pair of lines 3 feet from and parallel to the edge of a lane. Left and right wheel paths are based on the direction of travel.

Where IP testing is required, identify areas of localized roughness. Areas of localized roughness must be identified using the ProVAL smoothness assurance analysis by calculating continuous IRI for each wheel path with a 25-foot interval using a 250 mm filter.

Interpret references to "must-grinds" as "localized roughness" and "PI₀" as "MRI" in the RSS for section 39.

Replace the headings and paragraphs in section 39-1.12 with:

39-1.12A General

Section 39-1.12 includes specifications for measuring pavement smoothness with an inertial profiler (IP) and straightedge, analyzing the data with FHWA's engineering software ProVAL, and correcting deficient smoothness. Grinding equipment used for smoothness correction must comply with Section 42-3.03B.

Test pavement smoothness using an IP except use a 12-foot straightedge at the following locations:

- Traffic lanes less than 1,000 feet in length including ramps, turn lanes, and acceleration and deceleration lanes
- 2. HMA pavement within 3 feet from and parallel to the construction joint formed between curbs, gutters, or existing pavement

- 3. Areas within 15 feet of manholes
- 4. Shoulders
- 5. Weigh-in-motion areas
- 6. Miscellaneous areas such as medians, gore areas, turnouts, and maintenance pullouts

Where IP testing is required, pavement smoothness for each lane must be determined by the international roughness index (IRI) for the left and right wheel paths in an individual lane and then averaging the results. The average of the IRIs from the left and right wheel paths for the same lane is the mean roughness index (MRI) of the lane. The wheel paths are a pair of lines 3 feet from and parallel to the edge of a lane. Left and right wheel paths are based on the direction of travel.

Where IP testing is required, identify areas of localized roughness. Areas of localized roughness must be identified using the ProVAL smoothness assurance analysis by calculating continuous IRI for each wheel path with a 25-foot interval using a 250 mm filter.

Interpret references to "must-grinds" as "localized roughness" and " PI_0 " as "MRI" in the RSS for section 39.

39-1.12B Submittals

At least 5 business days before start of initial profiling or changing profiler or operator, submit:

- 1. IP certification issued by Texas Transportation Institute. The certification must be not more than 12 months old.
- Operator certification for the IP issued by Texas Transportation Institute. The certification must be not more than 36 months old.
- 3. List of manufacturer's recommended test procedures for IP calibration and verification.

Within 2 business days after cross correlation testing, submit ProVAL profiler certification analysis report for cross correlation test results performed on test section to the Engineer and to the electronic mailbox address:

smoothness@dot.ca.gov

Within 2 business days after each day of inertial profiling, submit profile data to the Engineer and to the electronic mailbox address:

smoothness@dot.ca.gov

Profiling data must include:

- 1. Raw profile data for each lane.
- 2. ProVAL ride quality analysis report for IRIs of left and right wheel paths of each lane. Submit in pdf file format
- 3. ProVAL ride quality analysis report for MRIs of each lane. Submit in pdf file format.
- 4. ProVAL smoothness assurance analysis report for IRIs of left wheel path. Submit in pdf file format.
- 5. ProVAL smoothness assurance analysis report for IRIs of right wheel path. Submit in pdf file format.
- 6. GPS data file for each lane in GPS exchange. Submit in GPS exchange file format.
- 7. Manufacturer's recommended IP calibration and verification tests results.
- 8. AASHTO IP calibration and verification test results including bounce, block, and distance measurement instrument (DMI).

Submit the raw profile data in unfiltered electronic pavement profile file (PPF) format. Name the PPF file using the following naming convention:

YYYYMMDD TTCCCRRR D L W S X PT.PPF

where:

YYYY = year

MM = Month, leading zero

DD = Day of month, leading zero

TT = District, leading zero

CCC = County, 2 or 3 letter abbreviation as shown in section 1-1.08

RRR = Route number, no leading zeros

D = Traffic direction as NB, SB, WB, or EB

L = Lane number from left to right in direction of travel

W = Wheel path as "L" for left, "R" for right, or "B" for both

- S = Beginning station to the nearest foot (i.e., 10+20) or beginning post mile to the nearest hundredth (i.e., 25.06) no leading zero
- X = Profile operation as "EXIST" for existing pavement, "INTER" for after prepaving smoothness correction, "PAVE" for after paving, and "CORR" for after final surface pavement correction

PT = Pavement type (i.e., HMA, RHMA, HMA-O, RHMA-O, RHMA-G, etc.)

Electronic PPF files that do not follow this standardized naming convention will be rejected.

Within 2 business days of performing straightedge measurements, submit areas requiring smoothness correction. Identify locations of smoothness correction by:

- 1. Location Number
- 2. District-County-Route
- 3. Beginning station or post mile to the nearest 0.01 mile
- 4. For correction areas within a lane:
 - 4.1. Lane direction as NB, SB, EB, or WB
 - 4.2. Lane number from left to right in direction of travel
 - 4.3. Wheel path as "L" for left, "R" for right, or "B" for both
- 5. For correction areas not within a lane:
 - 5.1. Identify pavement area (i.e., shoulder, weight station, turnout)
 - 5.2. Direction and distance from centerline as "L" for left or "R" for right
- 6. Estimated size of correction area

39-1.12C Inertial Profiler Calibration and Verification Tests

IP equipment must display a current certification decal with expiration date.

Operate the IP according to the manufacturer's recommendations and AASHTO R57-10 at 1-inch recording intervals.

Notify the Engineer 2 business days before performing IP calibration and verification testing.

Conduct the following IP calibration and verification tests in the Engineer's presence each day before performing inertial profiling:

- 1. Block test, Verify the height sensor accuracy under AASHTO R57-10, section 5.3.2.3.
- 2. Bounce test. Verify the combined height sensor and accelerometer accuracy under AASHTO R57-10, section 5.3.2.3.2.
- 3. DMI test. Calibrate the accuracy of the testing procedure under AASHTO R56-10, section 8.4.
- 4. Manufacturer's recommended tests.

Conduct cross correlation IP verification test in the Engineer's presence before performing initial profiling. Verify cross correlation IP verification test at least annually. Conduct 5 repeat runs of the IP on an authorized test section. The test section must be on an existing asphalt concrete pavement surface 0.1 mile long. Calculate a cross correlation to determine the repeatability of your device under Section 8.3.1.2 of AASHTO R56-10 using ProVAL profiler certification analysis with a 3 feet maximum offset. The cross correlation must be a minimum of 0.92.

For each 0.1 mile section, your IRI values must be within 10 percent of the Department's IRI values. The Engineer may order you to recalibrate your IP equipment and reprofile. If your results are inaccurate due to operator error, the Engineer may disqualify your IP operator.

39-1.12D Acceptance Criteria

For areas that require pavement smoothness determined using an IP, the pavement surface must:

- 1. Have no areas of localized roughness with an IRI greater than 120 in/mi
- 2. Comply with the MRI requirements shown in the following tables for a 0.1 mile section:

HMA^a Pavement Smoothness Acceptance Criteria

	•
HMA thickness	MRI requirement
> 0.20 foot	60 in/mi or less
≤0.20 foot	75 in/mi or less

^a Except OGFC

OGFC Pavement Smoothness Acceptance Criteria

OGFC placement on	MRI requirement
New construction, or HMA overlay	60 in/mi or less
Existing pavement	75 in/mi or less
Milled surface	75 in/mi or less

For areas that require pavement smoothness determined using a 12-foot straightedge, the HMA pavement surface must not vary from the lower edge of the straightedge by more than:

- 1. 0.01 foot when the straightedge is laid parallel with the centerline
- 0.02 foot when the straightedge is laid perpendicular to the centerline and extends from edge to edge of a traffic lane
- 3. 0.02 foot when the straightedge is laid within 24 feet of a pavement conform

Pavement smoothness may be accepted based on your testing in the absence of the Department's testing.

39-1.12E Smoothness Measurement

39-1.12E(1) General

Notify the Engineer of start location by station and start time at least 2 business days before profiling.

Remove foreign objects on the pavement surface before profiling.

39-1.12E(2) Inertial Profiler

Mark the beginning and ending station on the pavement shoulder before profiling. Stationing must be the same when profiling more than one surface.

While collecting the profile data to determine IRI, record the following locations in the raw profile data:

- 1. Begin and end of all bridge approach slabs
- 2. Begin and end of all bridges
- 3. Begin and end of all culverts visible on the roadway surface

Determine the MRI for 0.1-mile fixed sections using the ProVAL ride quality analysis with a 250 mm filter. Profile the left and right wheel paths of each lane. Calculate the MRI of each lane. A partial section less than 0.1 mile that is the result of an interruption to continuous pavement surface must comply with the MRI specifications for a full section. Adjust the MRI for a partial section to reflect a full section based on the proportion of a section paved.

Determine the areas of localized roughness using a continuous IRI for each wheel path with a 25-foot interval using a 250 mm filter. Localized roughness greater than 120 in/mi must be corrected regardless of the IRI values of a 0.1-mile section.

Determine the MRI of the HMA, except OGFC. If the MRI of the final pavement surface is greater than the MRI acceptance requirement in the table titled "HMA Pavement Smoothness Acceptance Criteria" in section 39-1.12D, correct to the MRI acceptance requirement in the table.

The final surface of HMA must meet MRI acceptance requirements in the table titled "HMA Pavement Smoothness Acceptance Criteria" in section 39-1.12D before placing OGFC.

Determine the MRI of the OGFC. If OGFC MRI is greater than the accepted value in the table titled "OGFC Pavement Smoothness Acceptance Criteria" in section 39-1.12D, correct to the MRI acceptance requirement in the table.

39-1.12E(3) Straightedge

Measure areas that require 12-foot straightedge. If the straightedge measurement is greater than the accepted value in section 39-1.12D, correct to the acceptance requirement.

39-1.12F Smoothness Correction

If the final surface of the pavement does not comply with section 39-1.12D, grind the pavement to within specified tolerances, remove and replace it, or place an overlay of HMA. Do not start corrective work until your method is authorized.

Smoothness correction of the final pavement surface must leave at least 75 percent of the specified HMA thickness. If ordered, core the pavement at the locations determined by the Engineer. Coring, including traffic control, is change order work. Remove and replace deficient pavement areas where the overlay thickness is less than 75 percent of the thickness specified as determined by the Engineer.

If you choose to correct OGFC, the Engineer determines if the corrective method causes raveling. OGFC that is raveling must be removed and replaced.

Corrected HMA pavement areas must be uniform rectangles with edges:

- 1. Parallel to the nearest HMA pavement edge or lane line
- 2. Perpendicular to the pavement centerline

On ground areas not to be overlaid with OGFC, apply fog seal coat under section 37-2.

Where corrections are made within areas requiring testing with IP, reprofile the entire lane length with the IP device.

Where corrections are made within areas requiring testing with a 12-foot straightedge, retest the corrected area with the straightedge.

39-1.12H Prepaying Grinding

Section 39-1.12H applies to existing asphalt concrete areas receiving an HMA overlay of less than or equal to 0.20 foot.

Correct areas of localized roughness greater than 140 in/mi.

Prepaving grinding day includes correcting areas of localized roughness, taking profiles of the corrected areas, and submitting profile data as specified in section 39-1.12B.

Notify the Engineer of those areas of localized roughness that cannot be corrected by prepaving grinding. The Engineer responds to your notification within 5 business days.

For those areas of localized roughness that cannot be corrected by grinding, the Engineer may order you to either (1) not correct the areas of localized roughness or (2) correct areas of localized roughness by a different method and take profiles of the corrected areas with an IP.

Corrective work not performed by prepaving grinding, including taking profiles of the corrected areas and associated traffic control, is change order work.

Correct prepaving areas of localized roughness that you predict will cause the final surface of HMA pavement to be noncompliant with the smoothness specifications. After correcting prepaving areas of localized roughness, take profiles of the corrected area and submit profile data as specified in section 39-1.12B.

Dispose of grinding residue.

Pave within 7 days of correcting areas.

The final pavement surface must comply with section 39-1.12D.

If ordered not to correct areas of localized roughness, the smoothness specifications do not apply to the final pavement surface placed in those areas.

Replace section 39-1.19 with:

39-1.19 HOT MIX ASPHALT AGGREGATE LIME TREATMENT—SLURRY METHOD

39-1.19A General

39-1.19A(1) Summary

Treat HMA aggregate with lime using the slurry method and place it in stockpiles to marinate.

39-1.19A(2) Submittals

Determine the exact lime proportions for treated aggregate stockpiles and resulting combined aggregate. Submit them as part of the proposed JMF.

Submit the averaged aggregate quality test results to the Engineer within 24 hours of sampling.

Submit a treatment data log from the slurry proportioning device in the following order:

- 1. Treatment date
- 2. Time of day the data is captured
- 3. Aggregate size being treated
- 4. Wet aggregate flow rate collected directly from the aggregate weigh belt
- 5. Moisture content of the aggregate just before treatment, expressed as a percent of the dry aggregate weight
- 6. Dry aggregate flow rate calculated from the wet aggregate flow rate
- 7. Lime slurry flow rate measured by the slurry meter
- 8. Dry lime flow rate calculated from the slurry meter output
- 9. Authorized lime ratio for each aggregate size being treated
- 10. Actual lime ratio calculated from the aggregate weigh belt and the slurry meter output, expressed as a percent of the dry aggregate weight
- 11. Calculated difference between the authorized lime ratio and the actual lime ratio
- 12. Dry lime and water proportions at the slurry treatment time

Every day during lime treatment, submit the treatment data log on electronic media in tab delimited format on a removable CD-ROM storage disk. Each continuous treatment data set must be a separate record using a line feed carriage return to present the specified data on 1 line. The reported data must include data titles at least once per report.

39-1.19A(3) Quality Control and Assurance

The QC plan must include aggregate quality control sampling and testing during aggregate lime treatment. Sample and test in compliance with frequencies in the following table:

Aggregate Quality Control During Lime Treatment

Quality characteristic	Test method	Minimum sampling and testing frequency
Sand equivalent	California Test 217	Once per 1,000 tons of aggregate treated with lime
Course durability index (D _c) (min) ^e	California Test 229	1 per 3,000 tons of
Fine durability index (D _f) (min)	California Test 229	aggregate treated with lime
Percent of crushed particles	California Test 205	
Los Angeles Rattler	California Test 211	As necessary and as
Fine aggregate angularity	California Test 234	designated in the QC plan
Flat and elongated particles	California Test 235	,

Note: During lime treatment, sample coarse and fine aggregate from individual stockpiles. Combine aggregate in the JMF proportions. Run tests for aggregate quality in triplicate and report test results as the average of 3 tests.

^eRequirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta County.

For any of the following, the Engineer orders proportioning operations stopped if you:

- 1. Do not submit the treatment data log
- 2. Do not submit the aggregate quality control data
- 3. Submit incomplete, untimely, or incorrectly formatted data
- 4. Do not take corrective actions
- 5. Take late or unsuccessful corrective actions
- 6. Do not stop treatment when proportioning tolerances are exceeded
- 7. Use malfunctioning or failed proportioning devices

If you stop treatment, notify the Engineer of any corrective actions taken and conduct a successful 20-minute test run before resuming treatment.

For the aggregate to be treated, determine the moisture content at least once during each 2 hours of treatment. Calculate moisture content under California Test 226 or 370 and report it as a percent of dry aggregate weight. Use the moisture content calculations as a set point for the proportioning process controller.

39-1.19B Materials

High-calcium hydrated lime and water must comply with section 24-2.02.

Before virgin aggregate is treated, it must comply with the aggregate quality specifications. Do not test treated aggregate for quality control except for gradation. The Engineer does not test treated aggregate for acceptance except for gradation.

The Engineer determines the combined aggregate gradation during HMA production after you have treated the aggregate. If RAP is used, the Engineer determines combined aggregate gradations containing RAP under Laboratory Procedure LP-9.

Treated aggregate must not have lime balls or clods.

39-1.19C Construction

39-1.19C(1) General

Notify the Engineer at least 24 hours before the start of aggregate treatment.

Treat aggregate separate from HMA production.

Do not treat RAP.

Add lime to the aggregate as slurry consisting of mixed dry lime and water at a ratio of 1 part lime to from 2 to 3 parts water by weight. The slurry must completely coat the aggregate.

Lime treat and marinate coarse and fine aggregate stockpiles separately.

Immediately before mixing lime slurry with the aggregate, water must not visibly separate from the aggregate.

Treat the aggregate and stockpile for marination only once.

The lime ratio is the pounds of dry hydrated lime per 100 lb of dry virgin aggregate expressed as a percentage. Water content of slurry or untreated aggregate must not affect the lime ratio.

The following aggregate gradations must have the lime ratio ranges shown in the following table:

Aggregate gradation	Lime ratio			
	percent			
Coarse virgin stockpiles a	0.4-1.0			
Fine virgin stockpiles ^a	1.5-2.0			
Combined virgin	1.0-1.5			
aggregate				

^a Stockpiles containing predominately coarse aggregate are coarse aggregate stockpiles. Stockpiles containing predominately fine aggregate are fine aggregate stockpiles.

For OGFC, you may reduce the combined virgin aggregate lime ratio to 0.5–1.0 percent.

The lime ratio for fine and coarse virgin aggregate stockpiles must be within ± 0.2 percent of the lime ratio in the accepted JMF. The lime ratio must be within ± 0.2 percent of the authorized lime ratio when you combine the individual aggregate sizes in the JMF proportions. The lime ratio must be determined before the addition of RAP.

If 3 consecutive sets of recorded treatment data indicate deviation more than 0.2 percent above or below the lime ratio in the accepted JMF, stop treatment.

If a set of recorded treatment data indicates a deviation of more than 0.4 percent above or below the lime ratio in the accepted JMF, stop treatment and do not use the material represented by that set of data in HMA.

If 20 percent or more of the total daily treatment indicates deviation of more than 0.2 percent above or below the lime ratio in the accepted JMF, stop treatment and do not use the day's total treatment in HMA.

If you stop treatment for noncompliance, you must implement corrective action and successfully treat aggregate for a 20-minute period. Notify the Engineer before beginning the 20-minute treatment period.

39-1.19C(2) Lime Slurry Proportioning

Proportion lime and water with a continuous or batch operation.

The device controlling slurry proportioning must produce a treatment data log. The log consists of a series of data sets captured at 10-minute intervals throughout daily treatment. The data must be a treatment activity register and not a summation. The material represented by the data set is the quantity produced 5 minutes before and 5 minutes after the capture time. For the Contract's duration, collected data must be stored by the controller.

39-1.19C(3) Proportioning and Mixing Lime Slurry Treated Aggregate

Treat HMA aggregate by proportioning lime slurry and aggregate by weight in a continuous operation.

Marinate treated aggregate in stockpiles from 24 hours to 60 days before using in HMA. Do not use aggregate marinated longer than 60 days.

39-1.19D Payment

Payment for treating aggregates with lime slurry is included in payment for the HMA involved.

Replace section 39-1.21 with:

39-1.21 REPLACE ASPHALT CONCRETE SURFACING

39-1.21A General

Remove existing asphalt concrete surfacing and underlying base and replace with HMA. The Engineer determines the exact limits of replaced asphalt concrete surfacing.

39-1.21B Materials

HMA for replace asphalt concrete surfacing must be Type A.

Asphalt binder for the HMA must be PG 64-28 M.

The aggregate for the HMA must comply with the 3/4 inch grading.

39-1.21C Construction

Place replacement HMA under section 39-3.

Replace asphalt concrete in a lane before the lane is specified to be opened to traffic under section 12-4.

Before removing asphalt concrete, outline the replacement area and cut neat lines with a saw or grind to full depth of the existing asphalt concrete. Do not damage asphalt concrete and base remaining in place.

Dispose of removed material.

If the base is excavated beyond the specified plane, replace it with HMA. The Department does not pay for this HMA.

39-1.21D Payment

Replace asphalt concrete surfacing is measured based on the specified dimensions and any adjustments ordered.

You may request authorization to leave rejected replacement HMA in place. If authorized, you must accept a reduction in the payment for the rejected replacement HMA.

Add to the first table of the RSS for section 39-2.02B:

Minimum Quality Control—Standard Construction Process

Quality	Test	Minimum	HMA type			
characteristic	method	sampling and testing frequency	Α	В	RHMA-G	OGFC
Coarse durability index (min) k, m	California Test 229	1 per 3,000 tons during production,	65	65	65	65
Fine durability index (min) k	California Test 229	but not less than 1 per paving day	50	50	50	50

^kObtain sample from stockpile before lime treatment.

Replace the 7th, 10th and 14th rows of the first table of the RSS for section 39-2.02B with:

Minimum Quality Control—Standard Construction Process

Quality characteristic	Test method	Minimum sampling and testing frequency	HMA type			
			Α	В	RHMA-G	OGFC
Air void content (%) ^{c, f}	California Test 367	1 per 4,000 tons or 2 per 5 business days, whichever is greater	4 ± 2	4 ± 2	3.5 ± 1.5	
Los Angeles Rattler (%, max) k Loss at 100 rev. Loss at 500 rev.	California Test 211	1 per 3,000 tons during production, but not less than 1 per paving day	12 25	 25	12 25	12 25
Voids in mineral aggregate(% min) i No. 4 grading 3/8" grading 1/2" grading 3/4" grading	California Test 367		17.0 15.0 14.0 13.0	17.0 15.0 14.0 13.0	 18.0–23.0 18.0–23.0	

Determine the following using AASHTO T84: bulk specific gravity (SSD) of fine aggregate, bulk specific gravity (oven dry) of fine aggregate, G_r , and absorption of fine aggregate.

The Requirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta County.

Add to the first table of the RSS for section 39-2.03A:

HMA Acceptance—Standard Construction Process

Quality	Test	HMA type				
characteristic	method	А	В	RHMA-G	OGFC	
Coarse durability index (min) j, l	California Test 229	65	65	65	65	
Fine durability index (min) i	California Test 229	50	50	50	50	

Obtain sample from stockpile before lime treatment.

Replace the 7th, 9th and 13th rows of the first table of the RSS for section 39-2.03A with:

HMA Acceptance—Standard Construction Process

Thin A Acceptance Changara Content action 1 100000						
Quality	Test	HMA type				
characteristic	method					
		Α	В	RHMA-G	OGFC	
Air void content	California	4 ± 2	4 ± 2	3.5 ± 1.5		
(%) ^{d, g}	Test 367					
Los Angeles	California					
Rattler (%, max)	Test 211					
Loss at 100 rev.		12		12	12	
Loss at 500 rev.		25	25	25	25	
Voids in mineral	California					
aggregate	Test 367 k					
(% min) ¹						
No. 4 grading		17.0	17.0			
3/8" grading		15.0	15.0			
1/2" grading		14.0	14.0	18.0–23.0		
3/4" grading		13.0	13.0	18.0-23.0		

^k Determine the following using AASHTO T84: bulk specific gravity (SSD) of fine aggregate, bulk specific gravity (oven dry) of fine aggregate, G_r, and absorption of fine aggregate.

aggregate.

Requirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta County.

Add to the first table of the RSS for section 39-3.02A:

HMA Acceptance—Method Construction Process

Quality	Test	HMA type				
characteristic	method	Α	В	RHMA-G	OGFC	
Coarse durability index (min) h, j	California Test 229	65	65	65	65	
Fine durability index (min) h	California Test 229	50	50	50	50	

h Obtain sample from stockpile before lime treatment.

Replace the 7th and 12th rows of the first table of the RSS for section 39-3.02A with:

HMA Acceptance—Method Construction Process

	***************************************	<u>u </u>			
Quality characteristic	Test	HMA type			
	method				
		Α	В	RHMA-G	OGFC
Los Angeles Rattler(%, max) h	California				
	Test 211				
Loss at 100 rev.		12		12	12
Loss at 500 rev.		25	25	25	25
Voids in mineral aggregate (%	California				
min) f	Test 367 i				
No. 4 grading		17.0	17.0		
3/8" grading		15.0	15.0		
1/2" grading		14.0	14.0	18.0-23.0	
3/4" grading		13.0	13.0	18.0-23.0	

Determine the following using AASHTO T84: bulk specific gravity (SSD) of fine aggregate, bulk specific gravity (oven dry) of fine aggregate, G_r, and absorption of fine aggregate.

^j Requirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta County.

Add to the first table of the RSS for section 39-4.02C:

Minimum Quality Control—QC/QA Construction Process

	willing daily control—QC/QA construction Process								
Quality characteristic	Test method	Minimum sampling and	HMA Type			Location of sampling	Maximum reporting time		
		testing frequency	Α	В	RHMA- G		allowance		
Coarse durability index(min) k, m	California Test 229	1 per 3,000 tons during	65	65	65	Stockpile	48 hours		
Fine Durability index (min) k	California Test 229	production , but not less than 1 per paving day	50	50	50	Stockpile	48 hours		

^{*}Obtain sample from stockpile before lime treatment.
Determine the following using AASHTO T84: bulk specific gravity (SSD) of fine aggregate, bulk specific gravity (oven dry) of fine aggregate, G_r, and absorption of fine aggregate.

^m Requirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta County.

Replace the 8th, 10th and 14th rows of the first table of the RSS for section 39-4.02C with:

Minimum Quality Control—QC/QA Construction Process

	Minimum Quality Control—QC/QA Construction Process							
Quality characteristic	Test method	Minimum sampling and testing frequency	НМА Туре			Location of sampling	Maximum reporting time allowance	
			А	В	RHMA-G			
Air void content (%) ^{f, g}	California Test 367	1 per 4,000 tons or 2 per 5 business days, whichever is greater	4 ± 2	4 ± 2	3.5 ± 1.5	Loose Mix Behind Paver See California Test 125	48 hours	
Los Angeles Rattler (% max) k: Loss at 100 rev. Loss at 500 rev.	California Test 211	1 per 3,000 tons during production, but not less than 1 per paving day	12 25	 25	12 25	Stockpile	48 hours	
Voids in mineral aggregate (% min.) i No. 4 grading 3/8" grading 1/2" grading 3/4" grading	California Test 367	As designated in QC plan. At least once per project.	17.0 15.0 14.0 13.0	17.0 15.0 14.0 13.0	 18.0–23.0 18.0–23.0	California Test 367	48 hours	

Add to the first table of the RSS for section 39-4.04A:

HMA Acceptance—QC/QA Construction Process

Index (i)	Quality characteristic	Weighting factor (w)	Test method	HMA type		
				Α	В	RHMA-G
	Coarse durability index (min) k, m		California Test 229	65	65	65
	Fine durability index(min) ^k		California Test 229	50	50	50

^kObtain sample from stockpile before lime treatment.

Replace the 6th, 9th and 12th rows of the first table of the RSS for section 39-4.04A with:

HMA Acceptance—QC/QA Construction Process

Index (i)	Quality characteristic	Weighting factor (w)	Test method	HMA type		
				Α	В	RHMA-G
	Air void content (%) ^{t, g}		California Test 367	4 ± 2	4 ± 2	3.5 ± 1.5
	Los Angeles Rattler (% max) k		California Test 211			
	Loss at 100 rev. Loss at 500 rev.			12 25	 25	12 25
	Voids in mineral aggregate (% min) i		California Test 367			
	No. 4 grading 3/8" grading			17.0 15.0	17.0 15.0	
	1/2" grading 3/4" grading			14.0 13.0	14.0 13.0	18.0–23.0 18.0–23.0

^^^^^^

DIVISION IX TRAFFIC CONTROL FACILITIES 83 RAILINGS AND BARRIERS

Replace section 83-1.02C(2)-83-1.02C(4) RESERVED with:

83-1.02C(3) Alternative Flared Terminal System

83-1.02C(3)(a) General

Alternative flared terminal system must be furnished and installed per the manufacturer's recommendations, and as described.

Determine the following using AASHTO T84: bulk specific gravity (SSD) of fine aggregate, bulk specific gravity (oven dry) of fine aggregate, G_r, and absorption of fine aggregate.

^m Requirement applies only if aggregate source is from Lassen, Modoc, Siskiyou or Shasta

County.

83-1.02C(3)(b) Materials

The allowable alternatives for a flared terminal system must consist of one of the following or a Department-authorized equal.

- TYPE FLEAT TERMINAL SYSTEM Type FLEAT terminal system must be a Flared Energy Absorbing Terminal 350 manufactured by Road Systems, Inc. The Flared Energy Absorbing Terminal 350 can be obtained from the distributor, Universal Industrial Sales, P.O. Box 699, Pleasant Grove, UT 84062, telephone (801) 785–0505 or from the distributor, Gregory Industries, Inc., 4100 13th Street, S.W., Canton, OH 44708, telephone (330) 477–4800.
- 2. TYPE SRT TERMINAL SYSTEM Type SRT terminal system must be an SRT-350 Slotted Rail Terminal (8-post system) as manufactured by Trinity Highway Products, LLC. The SRT-350 Slotted Rail Terminal (8-post system) can be obtained from the manufacturer, Trinity Highway Products, LLC, P.O. Box 99, Centerville, UT 84012, telephone (800) 772–7976.
- 3. TYPE X-TENSION TERMINAL SYSTEM Type X-tension terminal system must be an X-Tension Guard Rail End Terminal manufactured by Barrier Systems In. The Type X-Tension terminal system can be obtained from the manufacturer, Barrier Systems Inc., 3333 Vaca Valley Parkway Suite 800, Vacaville, CA 95688 telephone (707) 374-6800.

83-1.02C(3)(c) Submittals

Submit a certificate of compliance for terminal systems.

Submit at least two copies of the manufacturer's drawings, instruction manuals and maintenance manuals for each model of alternative flared terminal system to be used at least 10 days before installing terminal systems.

83-1.02C(3)(d) Construction

Each terminal system installed must be identified by painting the type of terminal system in neat black letters and figures 2 inches high on the backside of the rail element between system posts numbers 4 and 5. Apply a metallic acrylic resin type paint to a clean dry surface.

You must have a copy of the manufacturer's drawings, installation manual and maintenance manual for each model of terminal system on the job site during installation.

Install and backfill steel foundation tubes and soil tubes in conformance with section 83-1.02B.

If a soil plate is not used on steel foundation tube or soil tube, you must install $2 \times 5/8$ -inch bolts through the tube just below the bottom of the wood post to keep the post from dropping into the tube.

83-1.02C(3)(e) Payment

Not used

86 ELECTRICAL SYSTEMS

Add to the end of the 1st paragraph of section 86-1.01:

This work is shown on sheets labeled E. The work involved in each section 86 bid item is shown on a sheet with a sheet title matching the bid item description except for the following bid item:

1. Maintaining existing traffic management system elements during construction.

Add to section 86-1.03:

Submit a schedule of values within 15 days after Contract approval.

Add to the 4th paragraph of section 86-1.03:

13. Materials shown in the quantity tables on sheets labeled E.

Replace "Reserved" in section 86-1.06B with:

Traffic Management System (TMS) elements include, but are not limited to ramp metering (RM) system, communication system, traffic monitoring stations, video image vehicle detection system (VIVDS), microwave vehicle detection system (MVDS), loop detection system, changeable message sign (CMS) system, extinguishable message sign (EMS) system, highway advisory radio (HAR) system, closed circuit television (CCTV) camera system, roadway weather information system (RWIS), visibility sensor, and fiber optic system.

Existing TMS elements, including detection systems, shown and located within the project limits must remain in place and be protected from damage. If the construction activities require existing TMS elements to be nonoperational or off line, and if temporary or portable TMS elements are not shown, the Contractor must provide for temporary or portable TMS elements. The Contractor must receive authorization on the type of temporary or portable TMS elements and installation method.

Before work is performed, the Engineer, the Contractor, and the Department's Traffic Operations Electrical representatives must jointly conduct a pre-construction operational status check of all existing TMS elements and each element's communication status with the Traffic Management Center (TMC), including existing TMS elements not shown and elements that may not be impacted by the Contractor's activities. The Department's Traffic Operations Electrical representatives will certify the TMS elements' location and status, and provide a copy of the certified list of the existing TMS elements within the project limits to the Contractor. The status list will include the operational, defined as having full functionality, and the nonoperational components.

The Contractor must obtain authorization at least 72 hours before interrupting existing TMS elements' communication with the TMC that will result in the elements being nonoperational or off line. The Contractor must notify the Engineer at least 72 hours before starting excavation activities.

Traffic monitoring stations and their associated communication systems, which were verified to be operational during the pre-construction operational status check, must remain operational on freeway/highway mainline at all times, except:

- For a duration of up to 15 days on any continuous segment of the freeway/highway longer than 3
 miles
- For a duration of up to 60 days on any continuous segment of the freeway/highway shorter than 3 miles

If the construction activities require existing detection systems to be nonoperational or off line for a longer time period or the spacing between traffic monitoring stations is more than the specified criteria above, and temporary or portable detection operations are not shown, the Contractor must provide provisions for temporary or portable detection operations. The Contractor must receive authorization on the type of detection and installation before installing the temporary or portable detection.

If existing TMS elements shown or identified during the pre-construction operational status check, except traffic monitoring stations, are damaged or fail due to the Contractor's activity, where the elements are not fully functional, the Engineer must be notified immediately. If the Contractor is notified by the Engineer that existing TMS elements have been damaged, have failed or are not fully functional due to the Contractor's activity, the damaged or failed TMS elements, excluding structure-related elements, must be repaired or replaced, at the Contractor's expense, within 24 hours. For a structure-related elements, the Contractor must install temporary or portable TMS elements within 24 hours. For nonstructure-related TMS elements, the Engineer may authorize temporary or portable TMS elements for use during the construction activities.

The Contractor must demonstrate that repaired or replaced elements operate in a manner equal to or better than the replaced equipment. If the Contractor fails to perform required repairs or replacement work, the Department may perform the repair or replacement work and the cost will be deducted from monies due to the Contractor.

A TMS element must be considered nonoperational or off line for the duration of time that active communications with the TMC is disrupted, resulting in messages and commands not transmitted from or to the TMS element.

The Contractor must provide provisions for replacing existing TMS elements within the project limits, including detection systems, that were not identified on the plans or during the pre-construction operational status check that became damaged due to the Contractor's activities.

If the pre-construction operational status check identified existing TMS elements, then the Contractor, the Engineer, and the Department's Traffic Operations Electrical representatives must jointly conduct a post construction operational status check of all existing TMS elements and each element's communication status with the TMC. The Department's Traffic Operations Electrical representatives will certify the TMS elements' status and provide a copy of the certified list of the existing TMS elements within the project limits to the Contractor. The status list will include the operational, defined as having full functionality, and the nonoperational components. TMS elements that cease to be functional between pre and post construction status checks must be repaired at the Contractor's expense.

The Engineer will authorize the schedule for final replacement, the replacement methods and the replacement elements, including element types and installation methods before repair or replacement work is performed. The final TMS elements must be new and of equal or better quality than the existing TMS elements.

If no electrical work exists on the project and no TMS elements are identified within the project limits, the pre-construction operational status check is change order work.

Furnishing and installing temporary or portable TMS elements that are not shown, but are required when an existing TMS element becomes nonoperational or off line due to construction activities, is change order work.

Furnishing and installing temporary or portable TMS elements and replacing TMS elements that are not shown nor identified during the pre-construction operational status check and were damaged by construction activities is change order work.

If the Contractor is required to submit provisions for the replacement of TMS elements that were not identified, submitting the provisions is change order work.

Add to section 86-2.05A:

Conduit installed underground must be Type 1 or Type 3.

Add to section 86-2.05C:

If a standard coupling cannot be used for joining Type 1 conduit, use a UL-listed threaded union coupling under section 86-2.05C.

After conductors have been installed, the ends of the conduits must be sealed with an authorized type of sealing compound.

Add to section 86-2.08A:

Secure conductors and cables to the projecting end of the conduit in pull boxes.

Replace the 1st paragraph of section 86-2.09E with:

Splices must be insulated by "Method B."

Delete the 6th and 7th paragraphs of section 86-2.09E.

Delete the 8th paragraph of section 86-2.09E.

Replace section 86-2.18 with:

86-2.18 NUMBERING ELECTRICAL EQUIPMENT

The placement of numbers on electrical equipment will be done by others.

Replace 1st paragraph of section 86-2.18 with:

Add to section 86-5.01A(1):

Loop wire must be Type 2.

Loop detector lead-in cable must be Type B.

Slots must be filled with asphaltic emulsion sealant.

^^^^^^

DIVISION X MATERIALS 92 ASPHALTS

based on proposed RSS_92-1.02B_D07-10-12 from sponsor Replace the 2nd paragraph of section 92-1.02B with:

PG modified asphalt binder is asphalt binder modified with polymers, crumb rubber, or other additives, that must comply with the requirements shown in the following table:

PG Modified Asphalt Binder ^a

- Cambridge Aspirals 2000					
	AASHTO Test	Grade			
Property	Method	PG	PG	PG	
	IVICTIOU	58–34 M	64–28 M	76–22 M	
	Original Binder				
Flash point, min ℃	T 48	230	230	230	
Solubility, min % ^b	T 44 ^c	97.5	97.5	97.5	
Viscosity at 135°C ^d ,	T 040				
max, Pa·s	T 316	3.0	3.0	3.0	
Dynamic shear,					
Test temperature at 10 rad/s, °C	T 315	58	64	76	
min G*/sin(delta), kPa		1.00	1.00	1.00	
RTFO test ⁹ ,	T 240				
Mass loss, max, %	1 240	1.00	1.00	1.00	
RT	FO Test Aged Binder				
Dynamic shear,					
Test temperature at 10 rad/s, °C	T 315	58	64	76	
min G*/sin(delta), kPa		2.20	2.20	2.20	
Dynamic shear,					
Test temperature at 10 rad/s, ℃	T 315				
max (delta), degree		80 ^e	80 ^e	80 ^e	
Elastic recovery [†] ,					
Test temperature °C	T 301	25	25	25	
min recovery, %		75	75	65	
PAV ^h ,	R 28				
temperature, ℃		100	100	110	
RTFO Test and PAV Aged Binder					
Dynamic shear,					
Test temperature at 10 rad/s, ℃	T 315	16	22	31	
max G*sin(delta), kPa		5000	5000	5000	
Creep stiffness,					
Test temperature, ℃	T 313	-24	-18	-12	
max S-value, MPa		300	300	300	
min M-value		0.300	0.300	0.300	

^dThe Engineer waives this specification if the supplier provides written certification the asphalt can be adequately pumped and mixed at temperatures meeting applicable safety standards. ^eTest temperature is the temperature at which G*/sin(delta) is 2.2 kPa. A graph of log G*/sin(delta) plotted against temperature may be used to determine the test temperature when G*/sin(delta) is 2.2 kPa. A graph of (delta) versus temperature may be used to determine delta at the temperature when G*/sin(delta) is 2.2 kPa. The graph must have at least two points that envelope G*/sin(delta) of 2.2 kPa and the test temperature must not be more than 6 degree C apart. The Engineer also accepts direct measurement of (delta) at the temperature when

^fTests without a force ductility clamp may be performed.

^g"RTFO Test" means the asphaltic residue obtained using the Rolling Thin Film Oven Test, AASHTO Test Method T 240 or ASTM D 2872. The residue from mass change determination may be used for other tests.

G*/sin(delta) is 2.2 kPa.

Crumb rubber must be from automobile and truck tires, and free from contaminants including fabric, metal, and mineral or other nonrubber substances.

PG modified asphalt binders modified with crumb rubber must be homogeneous and must not contain visible particles of crumb rubber.

Supplier of PG modified asphalt binder modified with crumb rubber must certify:

- 1. The amount of crumb rubber by weight of asphalt binder
- 2. A minimum of 10 percent of crumb rubber by weight of asphalt binder

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^aDo not modify PG Polymer Modified using polyphosphoric acid modification.

^bThe Engineer waives this specification if the supplier is an Approved Supplier as defined by the Department's Certification Program for Suppliers of Asphalt.

^cThe Department allows ASTM D 5546 or ASTM D 7753 instead of AASHTO T 44. Particles recovered from ASTM D 5546 must be less than 250 μm.

h"PAV" means "Pressure Aging Vessel."

REVISED STANDARD SPECIFICATIONS APPLICABLE TO THE 2010 EDITION OF THE STANDARD SPECIFICATIONS

REVISED STANDARD SPECIFICATIONS DATED 02-22-13

Revised standard specifications are under headings that correspond with the main-section headings of the *Standard Specifications*. A main-section heading is a heading shown in the table of contents of the *Standard Specifications*. A date under a main-section heading is the date of the latest revision to the section.

Each revision to the *Standard Specifications* begins with a revision clause that describes a revision to the *Standard Specifications* or introduces a revision to the *Standard Specifications*. For a revision clause that describes a revision, the date on the right above the clause is the publication date of the revision. For a revision clause that introduces a revision, the date on the right above a revised term, phrase, clause, paragraph, or section is the publication date of the revised term, phrase, clause, paragraph or multiple-section revision, the date on the right above a paragraph or section is the publication date of the paragraphs or sections that follow.

Any paragraph added or deleted by a revision clause does not change the paragraph numbering of the *Standard Specifications* for any other reference to a paragraph of the *Standard Specifications*.

DIVISION I GENERAL PROVISIONS 1 GENERAL

10-19-12

Replace "current" in the 2nd paragraph of section 1-1.05 with:

04-20-12

most recent

Add to the 4th paragraph of section 1-1.05:

04-20-12

10-19-12

Any reference directly to a revised standard specification section is for convenience only. Lack of a direct reference to a revised standard specification section does not indicate a revised standard specification for the section does not exist.

Add to the 1st table in section 1-1.06:

	TRO	time-related overhead	
D	elete the abbr	eviation and its meaning for <i>UDBE</i> in the 1st table of section 1-1.06.	06-20-12
	Delete "C	ontract completion date" and its definition in section 1-1.07B.	10-19-12

10-19-12

Delete "critical delay" and its definition in section 1-1.07B.

Replace "day" and its definition in section 1-1.07B with:

10-19-12

day: 24 consecutive hours running from midnight to midnight; calendar day.

- 1. **business day:** Day on the calendar except a Saturday and a holiday.
- 2. **working day:** Time measure unit for work progress. A working day is any 24-consecutive-hour period except:
 - 2.1. Saturday and holiday.
 - 2.2. Day during which you cannot perform work on the controlling activity for at least 50 percent of the scheduled work shift with at least 50 percent of the scheduled labor and equipment due to any of the following:
 - 2.2.1. Adverse weather-related conditions.
 - 2.2.2. Maintaining traffic under the Contract.
 - 2.2.3. Suspension of a controlling activity that you and the Engineer agree benefits both parties.
 - 2.2.4. Unanticipated event not caused by either party such as:
 - 2.2.4.1. Act of God.
 - 2.2.4.2. Act of a public enemy.
 - 2.2.4.3. Epidemic.
 - 2.2.4.4. Fire.
 - 2.2.4.5. Flood.
 - 2.2.4.6. Governor-declared state of emergency.
 - 2.2.4.7. Landslide.
 - 2.2.4.8. Quarantine restriction.
 - 2.2.5. Issue involving a third party, including:
 - 2.2.5.1. Industry or area-wide labor strike.
 - 2.2.5.2. Material shortage.
 - 2.2.5.3. Freight embargo.
 - 2.2.5.4. Jurisdictional requirement of a law enforcement agency.
 - 2.2.5.5. Workforce labor dispute of a utility or nonhighway facility owner resulting in a nonhighway facility rearrangement not described and not solely for the Contractor's convenience. Rearrangement of a nonhighway facility includes installation, relocation, alteration, or removal of the facility.
 - 2.3. Day during a concurrent delay.
- 3. original working days:
 - 3.1. Working days to complete the work shown on the *Notice to Bidders* for a non–cost plus time based bid.
 - 3.2. Working days bid to complete the work for a cost plus time based bid.

Where working days is specified without the modifier "original" in the context of the number of working days to complete the work, interpret the number as the number of original working days as adjusted by any time adjustment.

Replace "Contract" in the definition of "early completion time" in section 1-1.07B with:

10-19-12

work

Replace "excusable delay" and its definition in section 1-1.07B with:

10-19-12

delay: Event that extends the completion of an activity.

- 1. **excusable delay:** Delay caused by the Department and not reasonably foreseeable when the work began such as:
 - 1.1. Change in the work
 - 1.2. Department action that is not part of the Contract

- 1.3. Presence of an underground utility main not described in the Contract or in a location substantially different from that specified
- 1.4. Described facility rearrangement not rearranged as described, by the utility owner by the date specified, unless the rearrangement is solely for the Contractor's convenience
- 1.5. Department's failure to obtain timely access to the right-of-way
- 1.6. Department's failure to review a submittal or provide notification in the time specified
- 2. critical delay: Excusable delay that extends the scheduled completion date
- 3. **concurrent delay:** Occurrence of at least 2 of the following events in the same period of time, either partially or entirely:
 - 3.1. Critical delay
 - 3.2. Delay to a controlling activity caused by you
 - 3.3. Non-working day

Replace "project" in the definition of "scheduled completion date" in section 1-1.07B with:

10-19-12

work

Add to section 1-1.07B:

10-19-12

Contract time: Number of original working days as adjusted by any time adjustment.

06-20-12

Disadvantaged Business Enterprise: Disadvantaged Business Enterprise as defined in 49 CFR 26.5.

Replace "PO BOX 911" in the District 3 mailing address in the table in section 1-1.08 with:

04-20-12

703 B ST

Add to the table in section 1-1.11:

		01-20-12
Projects Currently	a.gov/hq/esc/oe/	
Advertised	weekly_ads/all_ advertised.php	

^^^^^

2 BIDDING

10-19-12

Replace the 3rd paragraph of section 2-1.06B with:

01-20-12

If an *Information Handout* or cross sections are available:

- 1. You may view them at the Contract Plans and Special Provisions link at the Office Engineer–All Projects Currently Advertised Web site
- 2. For an informal-bid contract, you may obtain them at the Bidders' Exchange street address

Add a paragraph break between the 1st and 2nd sentences of the 5th paragraph of section 2-1.06B.

Add between "and" and "are" in item 2 in the list in the 7th paragraph of section 2-1.06B:

04-20-12

they

06-20-12

Delete "Underutilized" in "Underutilized Disadvantaged Business Enterprises" in the heading of section 2-1.12B.

06-20-12

Delete *U* in *UDBE* at each occurrence in section 2-1.12B.

Replace the 2nd paragraph of section 2-1.12B(1) with:

06-20-12

To ensure equal participation of DBEs provided in 49 CFR 26.5, the Department shows a goal for DBEs.

06-20-12

Delete the 3rd paragraph of section 2-1.12B(1):

Replace the 7th paragraph of section 2-1.12B(1) with:

06-20-12

All DBE participation will count toward the Department's federally-mandated statewide overall DBE goal.

Replace "offered" at the end of the 2nd sentence of item 7 in the list of 2nd paragraph of section 2-1.12B(3) with:

06-20-12

provided

01-20-12

Delete the 2nd paragraph of section 2-1.33A.

Replace the 3rd paragraph of section 2-1.33A with:

01-20-12

Except for each subcontracted bid item number and corresponding percentage and proof of each required SSPC QP certification, do not fax submittals.

Add to section 2-1.33C:

10-19-12

On the *Subcontractor List*, you must either submit each subcontracted bid item number and corresponding percentage with your bid or fax these numbers and percentages to (916) 227-6282 within 24 hours after bid opening. Failure to do so results in a nonresponsive bid.

Replace the paragraph in section 2-1.35 with:

01-20-12

Submit proof of each required SSPC QP certification with your bid or fax it to (916) 227-6282 no later than 4:00 p.m. on the 2nd business day after bid opening. Failure to do so results in a nonresponsive bid.

3 CONTRACT AWARD AND EXECUTION

10-19-12

Add to the end of section 3-1.04:

10-19-12

You may request to extend the award period by faxing a request to (916) 227-6282 before 4:00 p.m. on the last day of the award period. If you do not make this request, after the specified award period:

- 1. Your bid becomes invalid
- 2. You are not eligible for the award of the contract

Replace the paragraph in section 3-1.11 with:

10-19-12

Complete and deliver to the Office Engineer a Payee Data Record when requested by the Department.

Replace section 3-1.13 with:

07-27-12

3-1.13 FORM FHWA-1273

For a federal-aid contract, form FHWA-1273 is included with the Contract form in the documents sent to the successful bidder for execution. Comply with its provisions. Interpret the training and promotion section as specified in section 7-1.11A.

Add to item 1 in the list in the 2nd paragraph of section 3-1.18:

, including the attached form FHWA-1273

07-27-12

10-19-12

Delete item 4 of the 2nd paragraph of section 3-1.18.

^^^^^^

5 CONTROL OF WORK

10-19-12

Add between "million" and ", professionally" in the 3rd paragraph of section 5-1.09A:

10-19-12

and 100 or more working days

Add to the list in the 4th paragraph of section 5-1.09A:

9. Considering discussing with and involving all stakeholders in evaluating potential VECPs

10-19-12

Add to the end of item 1.1 in the list in the 7th paragraph of section 5-1.09A:

, including VECPs

10-19-12

Replace the 1st paragraph of section 5-1.09C with:

10-19-12

For a contract with a total bid over \$10 million and 100 or more working days, training in partnering skills development is required.

10-19-12

Delete the 2nd paragraph of section 5-1.09C.

Replace "at least 2 representatives" in the 5th paragraph of section 5-1.09C with:

10-19-12

field supervisory personnel

Replace the 1st and 2nd sentences in the 7th paragraph of section 5-1.13B(1) with:

If a DBE is decertified before completing its work, the DBE must notify you in writing of the decertification date. If a business becomes a certified DBE before completing its work, the business must notify you in writing of the certification date.

Replace "90" in the last sentence of the 7th paragraph of section 5-1.13B(1) with:

06-20-12

30

Replace "Underutilized" in "Underutilized Disadvantaged Business Enterprises" in the heading of section 5-1.13B(2) with:

Performance of

06-20-12

Delete *U* in *UDBE* at each occurrence in section 5-1.13B(2).

06-20-12

Replace the 3rd paragraph of section 5-1.13B(2) with:

06-20-12

Do not terminate or substitute a listed DBE for convenience and perform the work with your own forces or obtain materials from other sources without authorization from the Department.

Replace item 6 in the list in the 4th paragraph of section 5-1.13B(2) with:

06-20-12

6. Listed DBE is ineligible to work on the project because of suspension or debarment.

Add to the list in the 4th paragraph of section 5-1.13B(2):

06-20-12

- 8. Listed DBE voluntarily withdraws with written notice from the Contract.
- 9. Listed DBE is ineligible to receive credit for the type of work required.
- Listed DBE owner dies or becomes disabled resulting in the inability to perform the work on the Contract.
- 11. Department determines other documented good cause.

Add between the 4th and 5th paragraphs of section 5-1.13B(2):

07-20-12

Notify the original DBE of your intent to use other forces or material sources and provide the reasons. Provide the DBE with 5 days to respond to your notice and advise you and the Department of the reasons why the use of other forces or sources of materials should not occur. Your request to use other forces or material sources must include:

- 1. 1 or more of the reasons listed in the preceding paragraph
- 2. Notices from you to the DBE regarding the request
- 3. Notices from the DBE to you regarding the request

Add between "terminated" and ", you" in the 5th paragraph of section 5-1.13B(2):

07-20-12

or substituted

Replace "Contract" in item 1 in the list in the 5th paragraph of section 5-1.13C with:

10-19-12

work

Replace "Reserved" in section 5-1.20C with:

10-19-12

If the Contract includes an agreement with a railroad company, the Department makes the provisions of the agreement available in the *Information Handout* in the document titled "Railroad Relations and Insurance Requirements." Comply with the requirements in the document.

Add between the 2nd and 3rd paragraphs of section 5-1.23A:

10-19-12

Submit action and informational submittals to the Engineer.

Add to section 5-1.36C:

07-20-12

If the Contract does not include an agreement with a railroad company, do not allow personnel or equipment on railroad property.

Prevent material, equipment, and debris from falling onto railroad property.

Add between the 1st and 2nd paragraphs of section 5-1.37A:

10-19-12

Do not remove any padlock used to secure a portion of the work until the Engineer is present to replace it. Notify the Engineer at least 3 days before removing the lock.

Replace the 1st sentence of the 1st paragraph of section 5-1.39C(2) with:

10-19-12

Section 5-1.39C(2) applies if a plant establishment period of 3 years or more is shown on the *Notice to Bidders*.

Replace "working days" in the 1st paragraph of section 5-1.43E(1)(a) with:

10-19-12

original working days

^^^^^

7 LEGAL RELATIONS AND RESPONSIBILITY TO THE PUBLIC

07-27-12

Replace "20 days" in the 14th paragraph of section 7-1.04 with:

09-16-11

25 days

Replace "90 days" in the 14th paragraph of section 7-1.04 with:

09-16-11

125 days

Add between the 18th and 19th paragraphs of section 7-1.04:

09-16-11

Temporary facilities that could be a hazard to public safety if improperly designed must comply with design requirements described in the Contract for those facilities or, if none are described, with standard design criteria or codes appropriate for the facility involved. Submit shop drawings and design calculations for the temporary facilities and show the standard design criteria or codes used. Shop drawings and supplemental calculations must be sealed and signed by an engineer who is registered as a civil engineer in the State.

Replace the 2nd paragraph of section 7-1.11A with:

07-27-12

A copy of form FHWA-1273 is included in section 7-1.11B. The training and promotion section of section II refers to training provisions as if they were included in the special provisions. The Department specifies the provisions in section 7-1.11D of the *Standard Specifications*. If a number of trainees or apprentices is required, the Department shows the number on the *Notice to Bidders*. Interpret each FHWA-1273 clause shown in the following table as having the same meaning as the corresponding Department clause:

FHWA-1273 Nondiscrimination Clauses

FHWA-1273 section	FHWA-1273 clause	Department clause
Training and Promotion	In the event a special provision for training is provided under this contract, this subparagraph will be superseded as indicated in the special provision.	If section 7-1.11D applies, section 7-1.11D supersedes this subparagraph.
Records and Reports	If on-the-job training is being required by special provision, the contractor will be required to collect and report training data.	If the Contract requires on-the- job training, collect and report training data.

Replace the form in section 7-1.11B with:

07-20-12

REQUIRED CONTRACT PROVISIONS FEDERAL-AID CONSTRUCTION CONTRACTS

- General
- II. Nondiscrimination
- III. Nonsegregated Facilities
- IV. Davis-Bacon and Related Act Provisions
- Contract Work Hours and Safety Standards Act Provisions
- VI. Subletting or Assigning the Contract
- VII. Safety: Accident Prevention
- VIII. False Statements Concerning Highway Projects
- IX. Implementation of Clean Air Act and Federal Water Pollution Control Act
- Compliance with Governmentwide Suspension and Debarment Requirements
- Certification Regarding Use of Contract Funds for Lobbying

ATTACHMENTS

A. Employment and Materials Preference for Appalachian Development Highway System or Appalachian Local Access Road Contracts (included in Appalachian contracts only)

I. GENERAL

 Form FHWA-1273 must be physically incorporated in each construction contract funded under Title 23 (excluding emergency contracts solely intended for debris removal). The contractor (or subcontractor) must insert this form in each subcontract and further require its inclusion in all lower tier subcontracts (excluding purchase orders, rental agreements and other agreements for supplies or services).

The applicable requirements of Form FHWA-1273 are incorporated by reference for work done under any purchase order, rental agreement or agreement for other services. The prime contractor shall be responsible for compliance by any subcontractor, lower-tier subcontractor or service provider.

Form FHWA-1273 must be included in all Federal-aid designbuild contracts, in all subcontracts and in lower tier subcontracts (excluding subcontracts for design services, purchase orders, rental agreements and other agreements for supplies or services). The design-builder shall be responsible for compliance by any subcontractor, lower-tier subcontractor or service provider.

Contracting agencies may reference Form FHWA-1273 in bid proposal or request for proposal documents, however, the Form FHWA-1273 must be physically incorporated (not referenced) in all contracts, subcontracts and lower-tier subcontracts (excluding purchase orders, rental agreements and other agreements for supplies or services related to a construction contract).

 Subject to the applicability criteria noted in the following sections, these contract provisions shall apply to all work performed on the contract by the contractor's own organization and with the assistance of workers under the contractor's immediate superintendence and to all work performed on the contract by piecework, station work, or by subcontract.

- A breach of any of the stipulations contained in these Required Contract Provisions may be sufficient grounds for withholding of progress payments, withholding of final payment, termination of the contract, suspension / debarment or any other action determined to be appropriate by the contracting agency and FHWA.
- 4. Selection of Labor: During the performance of this contract, the contractor shall not use convict labor for any purpose within the limits of a construction project on a Federal-aid highway unless it is labor performed by convicts who are on parole, supervised release, or probation. The term Federal-aid highway does not include roadways functionally classified as local roads or rural minor collectors.

II. NONDISCRIMINATION

The provisions of this section related to 23 CFR Part 230 are applicable to all Federal-aid construction contracts and to all related construction subcontracts of \$10,000 or more. The provisions of 23 CFR Part 230 are not applicable to material supply, engineering, or architectural service contracts.

In addition, the contractor and all subcontractors must comply with the following policies: Executive Order 11246, 41 CFR 60, 29 CFR 1625-1627, Title 23 USC Section 140, the Rehabilitation Act of 1973, as amended (29 USC 794), Title VI of the Civil Rights Act of 1964, as amended, and related regulations including 49 CFR Parts 21, 26 and 27; and 23 CFR Parts 200, 230, and 633.

The contractor and all subcontractors must comply with: the requirements of the Equal Opportunity Clause in 41 CFR 60-1.4(b) and, for all construction contracts exceeding \$10,000, the Standard Federal Equal Employment Opportunity Construction Contract Specifications in 41 CFR 60-4.3.

Note: The U.S. Department of Labor has exclusive authority to determine compliance with Executive Order 11246 and the policies of the Secretary of Labor including 41 CFR 60, and 29 CFR 1625-1627. The contracting agency and the FHWA have the authority and the responsibility to ensure compliance with Title 23 USC Section 140, the Rehabilitation Act of 1973, as amended (29 USC 794), and Title VI of the Civil Rights Act of 1964, as amended, and related regulations including 49 CFR Parts 21, 26 and 27; and 23 CFR Parts 200, 230, and 633.

The following provision is adopted from 23 CFR 230, Appendix A, with appropriate revisions to conform to the U.S. Department of Labor (US DOL) and FHWA requirements.

1. Equal Employment Opportunity: Equal employment opportunity (EEO) requirements not to discriminate and to take affirmative action to assure equal opportunity as set forth under laws, executive orders, rules, regulations (28 CFR 35, 29 CFR 1630, 29 CFR 1625-1627, 41 CFR 60 and 49 CFR 27) and orders of the Secretary of Labor as modified by the provisions prescribed herein, and imposed pursuant to 23 U.S.C. 140 shall constitute the EEO and specific affirmative action standards for the contractor's project activities under

this contract. The provisions of the Americans with Disabilities Act of 1990 (42 U.S.C. 12101 et seq.) set forth under 28 CFR 35 and 29 CFR 1630 are incorporated by reference in this contract. In the execution of this contract, the contractor agrees to comply with the following minimum specific requirement activities of EEO:

- a. The contractor will work with the contracting agency and the Federal Government to ensure that it has made every good faith effort to provide equal opportunity with respect to all of its terms and conditions of employment and in their review of activities under the contract.
- b. The contractor will accept as its operating policy the following statement:

"It is the policy of this Company to assure that applicants are employed, and that employees are treated during employment, without regard to their race, religion, sex, color, national origin, age or disability. Such action shall include: employment, upgrading, demotion, or transfer; recruitment or recruitment advertising; layoff or termination; rates of pay or other forms of compensation; and selection for training, including apprenticeship, pre-apprenticeship, and/or on-the-job training."

- 2. EEO Officer: The contractor will designate and make known to the contracting officers an EEO Officer who will have the responsibility for and must be capable of effectively administering and promoting an active EEO program and who must be assigned adequate authority and responsibility to do so
- 3. Dissemination of Policy: All members of the contractor's staff who are authorized to hire, supervise, promote, and discharge employees, or who recommend such action, or who are substantially involved in such action, will be made fully cognizant of, and will implement, the contractor's EEO policy and contractual responsibilities to provide EEO in each grade and classification of employment. To ensure that the above agreement will be met, the following actions will be taken as a minimum:
- a. Periodic meetings of supervisory and personnel office employees will be conducted before the start of work and then not less often than once every six months, at which time the contractor's EEO policy and its implementation will be reviewed and explained. The meetings will be conducted by the EEO Officer.
- b. All new supervisory or personnel office employees will be given a thorough indoctrination by the EEO Officer, covering all major aspects of the contractor's EEO obligations within thirty days following their reporting for duty with the contractor.
- c. All personnel who are engaged in direct recruitment for the project will be instructed by the EEO Officer in the contractor's procedures for locating and hiring minorities and women.
- d. Notices and posters setting forth the contractor's EEO policy will be placed in areas readily accessible to employees, applicants for employment and potential employees.
- e. The contractor's EEO policy and the procedures to implement such policy will be brought to the attention of employees by means of meetings, employee handbooks, or other appropriate means.

- 4. Recruitment: When advertising for employees, the contractor will include in all advertisements for employees the notation: "An Equal Opportunity Employer." All such advertisements will be placed in publications having a large circulation among minorities and women in the area from which the project work force would normally be derived.
- a. The contractor will, unless precluded by a valid bargaining agreement, conduct systematic and direct recruitment through public and private employee referral sources likely to yield qualified minorities and women. To meet this requirement, the contractor will identify sources of potential minority group employees, and establish with such identified sources procedures whereby minority and women applicants may be referred to the contractor for employment consideration.
- b. In the event the contractor has a valid bargaining agreement providing for exclusive hiring hall referrals, the contractor is expected to observe the provisions of that agreement to the extent that the system meets the contractor's compliance with EEO contract provisions. Where implementation of such an agreement has the effect of discriminating against minorities or women, or obligates the contractor to do the same, such implementation violates Federal nondiscrimination provisions.
- c. The contractor will encourage its present employees to refer minorities and women as applicants for employment. Information and procedures with regard to referring such applicants will be discussed with employees.
- 5. Personnel Actions: Wages, working conditions, and employee benefits shall be established and administered, and personnel actions of every type, including hiring, upgrading, promotion, transfer, demotion, layoff, and termination, shall be taken without regard to race, color, religion, sex, national origin, age or disability. The following procedures shall be followed:
- a. The contractor will conduct periodic inspections of project sites to insure that working conditions and employee facilities do not indicate discriminatory treatment of project site personnel.
- b. The contractor will periodically evaluate the spread of wages paid within each classification to determine any evidence of discriminatory wage practices.
- c. The contractor will periodically review selected personnel actions in depth to determine whether there is evidence of discrimination. Where evidence is found, the contractor will promptly take corrective action. If the review indicates that the discrimination may extend beyond the actions reviewed, such corrective action shall include all affected persons.
- d. The contractor will promptly investigate all complaints of alleged discrimination made to the contractor in connection with its obligations under this contract, will attempt to resolve such complaints, and will take appropriate corrective action within a reasonable time. If the investigation indicates that the discrimination may affect persons other than the complainant, such corrective action shall include such other persons. Upon completion of each investigation, the contractor will inform every complainant of all of their avenues of appeal.

6. Training and Promotion:

a. The contractor will assist in locating, qualifying, and increasing the skills of minorities and women who are

applicants for employment or current employees. Such efforts should be aimed at developing full journey level status employees in the type of trade or job classification involved.

- b. Consistent with the contractor's work force requirements and as permissible under Federal and State regulations, the contractor shall make full use of training programs, i.e., apprenticeship, and on-the-job training programs for the geographical area of contract performance. In the event a special provision for training is provided under this contract, this subparagraph will be superseded as indicated in the special provision. The contracting agency may reserve training positions for persons who receive welfare assistance in accordance with 23 U.S.C. 140(a).
- c. The contractor will advise employees and applicants for employment of available training programs and entrance requirements for each.
- d. The contractor will periodically review the training and promotion potential of employees who are minorities and women and will encourage eligible employees to apply for such training and promotion.
- 7. Unions: If the contractor relies in whole or in part upon unions as a source of employees, the contractor will use good faith efforts to obtain the cooperation of such unions to increase opportunities for minorities and women. Actions by the contractor, either directly or through a contractor's association acting as agent, will include the procedures set forth below:
- a. The contractor will use good faith efforts to develop, in cooperation with the unions, joint training programs aimed toward qualifying more minorities and women for membership in the unions and increasing the skills of minorities and women so that they may qualify for higher paying employment.
- b. The contractor will use good faith efforts to incorporate an EEO clause into each union agreement to the end that such union will be contractually bound to refer applicants without regard to their race, color, religion, sex, national origin, age or disability.
- c. The contractor is to obtain information as to the referral practices and policies of the labor union except that to the extent such information is within the exclusive possession of the labor union and such labor union refuses to furnish such information to the contractor, the contractor shall so certify to the contracting agency and shall set forth what efforts have been made to obtain such information.
- d. In the event the union is unable to provide the contractor with a reasonable flow of referrals within the time limit set forth in the collective bargaining agreement, the contractor will, through independent recruitment efforts, fill the employment vacancies without regard to race, color, religion, sex, national origin, age or disability; making full efforts to obtain qualified and/or qualifiable minorities and women. The failure of a union to provide sufficient referrals (even though it is obligated to provide exclusive referrals under the terms of a collective bargaining agreement) does not relieve the contractor from the requirements of this paragraph. In the event the union referral practice prevents the contractor from meeting the obligations pursuant to Executive Order 11246, as amended, and these special provisions, such contractor shall immediately notify the contracting agency.
- 8. Reasonable Accommodation for Applicants / Employees with Disabilities: The contractor must be familiar

- with the requirements for and comply with the Americans with Disabilities Act and all rules and regulations established there under. Employers must provide reasonable accommodation in all employment activities unless to do so would cause an undue hardship.
- 9. Selection of Subcontractors, Procurement of Materials and Leasing of Equipment: The contractor shall not discriminate on the grounds of race, color, religion, sex, national origin, age or disability in the selection and retention of subcontractors, including procurement of materials and leases of equipment. The contractor shall take all necessary and reasonable steps to ensure nondiscrimination in the administration of this contract.
- The contractor shall notify all potential subcontractors and suppliers and lessors of their EEO obligations under this contract
- b. The contractor will use good faith efforts to ensure subcontractor compliance with their EEO obligations.

10. Assurance Required by 49 CFR 26.13(b):

- a. The requirements of 49 CFR Part 26 and the State DOT's U.S. DOT-approved DBE program are incorporated by reference.
- b. The contractor or subcontractor shall not discriminate on the basis of race, color, national origin, or sex in the performance of this contract. The contractor shall carry out applicable requirements of 49 CFR Part 26 in the award and administration of DOT-assisted contracts. Failure by the contractor to carry out these requirements is a material breach of this contract, which may result in the termination of this contract or such other remedy as the contracting agency deems appropriate.
- 11. Records and Reports: The contractor shall keep such records as necessary to document compliance with the EEO requirements. Such records shall be retained for a period of three years following the date of the final payment to the contractor for all contract work and shall be available at reasonable times and places for inspection by authorized representatives of the contracting agency and the FHWA.
- a. The records kept by the contractor shall document the following:
- The number and work hours of minority and nonminority group members and women employed in each work classification on the project;
 - (2) The progress and efforts being made in cooperation with unions, when applicable, to increase employment opportunities for minorities and women; and
- (3) The progress and efforts being made in locating, hiring, training, qualifying, and upgrading minorities and women;
- b. The contractors and subcontractors will submit an annual report to the contracting agency each July for the duration of the project, indicating the number of minority, women, and non-minority group employees currently engaged in each work classification required by the contract work. This information is to be reported on Form FHWA-1391. The staffing data should represent the project work force on board in all or any part of the last payroll period preceding the end of July. If on-the-job training is being required by special provision, the contractor

will be required to collect and report training data. The employment data should reflect the work force on board during all or any part of the last payroll period preceding the end of July.

III. NONSEGREGATED FACILITIES

This provision is applicable to all Federal-aid construction contracts and to all related construction subcontracts of \$10,000 or more.

The contractor must ensure that facilities provided for employees are provided in such a manner that segregation on the basis of race, color, religion, sex, or national origin cannot result. The contractor may neither require such segregated use by written or oral policies nor tolerate such use by employee custom. The contractor's obligation extends further to ensure that its employees are not assigned to perform their services at any location, under the contractor's control, where the facilities are segregated. The term "facilities" includes waiting rooms, work areas, restaurants and other eating areas, time clocks, restrooms, washrooms, locker rooms, and other storage or dressing areas, parking lots, drinking fountains, recreation or entertainment areas, transportation, and housing provided for employees. The contractor shall provide separate or single-user restrooms and necessary dressing or sleeping areas to assure privacy between sexes.

IV. DAVIS-BACON AND RELATED ACT PROVISIONS

This section is applicable to all Federal-aid construction projects exceeding \$2,000 and to all related subcontracts and lower-tier subcontracts (regardless of subcontract size). The requirements apply to all projects located within the right-of-way of a roadway that is functionally classified as Federal-aid highway. This excludes roadways functionally classified as local roads or rural minor collectors, which are exempt. Contracting agencies may elect to apply these requirements to other projects.

The following provisions are from the U.S. Department of Labor regulations in 29 CFR 5.5 "Contract provisions and related matters" with minor revisions to conform to the FHWA-1273 format and FHWA program requirements.

1. Minimum wages

a. All laborers and mechanics employed or working upon the site of the work, will be paid unconditionally and not less often than once a week, and without subsequent deduction or rebate on any account (except such payroll deductions as are permitted by regulations issued by the Secretary of Labor under the Copeland Act (29 CFR part 3)), the full amount of wages and bona fide fringe benefits (or cash equivalents thereof) due at time of payment computed at rates not less than those contained in the wage determination of the Secretary of Labor which is attached hereto and made a part hereof, regardless of any contractual relationship which may be alleged to exist between the contractor and such laborers and mechanics.

Contributions made or costs reasonably anticipated for bona fide fringe benefits under section 1(b)(2) of the Davis-Bacon Act on behalf of laborers or mechanics are considered wages paid to such laborers or mechanics, subject to the provisions

of paragraph 1.d. of this section; also, regular contributions made or costs incurred for more than a weekly period (but not less often than quarterly) under plans, funds, or programs which cover the particular weekly period, are deemed to be constructively made or incurred during such weekly period. Such laborers and mechanics shall be paid the appropriate wage rate and fringe benefits on the wage determination for the classification of work actually performed, without regard to skill, except as provided in 29 CFR 5.5(a)(4). Laborers or mechanics performing work in more than one classification may be compensated at the rate specified for each classification for the time actually worked therein: Provided, That the employer's payroll records accurately set forth the time spent in each classification in which work is performed The wage determination (including any additional classification and wage rates conformed under paragraph 1.b. of this section) and the Davis-Bacon poster (WH-1321) shall be posted at all times by the contractor and its subcontractors at the site of the work in a prominent and accessible place where it can be easily seen by the workers.

- b.(1) The contracting officer shall require that any class of laborers or mechanics, including helpers, which is not listed in the wage determination and which is to be employed under the contract shall be classified in conformance with the wage determination. The contracting officer shall approve an additional classification and wage rate and fringe benefits therefore only when the following criteria have been met:
 - (i) The work to be performed by the classification requested is not performed by a classification in the wage determination: and
 - (ii) The classification is utilized in the area by the construction industry; and
 - (iii) The proposed wage rate, including any bona fide fringe benefits, bears a reasonable relationship to the wage rates contained in the wage determination.
- (2) If the contractor and the laborers and mechanics to be employed in the classification (if known), or their representatives, and the contracting officer agree on the classification and wage rate (including the amount designated for fringe benefits where appropriate), a report of the action taken shall be sent by the contracting officer to the Administrator of the Wage and Hour Division, Employment Standards Administration, U.S. Department of Labor, Washington, DC 20210. The Administrator, or an authorized representative, will approve, modify, or disapprove every additional classification action within 30 days of receipt and so advise the contracting officer or will notify the contracting officer within the 30-day period that additional time is necessary.
- (3) In the event the contractor, the laborers or mechanics to be employed in the classification or their representatives, and the contracting officer do not agree on the proposed classification and wage rate (including the amount designated for fringe benefits, where appropriate), the contracting officer shall refer the questions, including the views of all interested parties and the recommendation of the contracting officer, to the Wage and Hour Administrator for determination. The Wage and Hour Administrator, or an authorized representative, will issue a determination within 30 days of receipt and so advise the contracting officer or

will notify the contracting officer within the 30-day period that additional time is necessary.

- (4) The wage rate (including fringe benefits where appropriate) determined pursuant to paragraphs 1.b.(2) or 1.b.(3) of this section, shall be paid to all workers performing work in the classification under this contract from the first day on which work is performed in the classification.
- c. Whenever the minimum wage rate prescribed in the contract for a class of laborers or mechanics includes a fringe benefit which is not expressed as an hourly rate, the contractor shall either pay the benefit as stated in the wage determination or shall pay another bona fide fringe benefit or an hourly cash equivalent thereof.
- d. If the contractor does not make payments to a trustee or other third person, the contractor may consider as part of the wages of any laborer or mechanic the amount of any costs reasonably anticipated in providing bona fide fringe benefits under a plan or program, Provided, That the Secretary of Labor has found, upon the written request of the contractor, that the applicable standards of the Davis-Bacon Act have been met. The Secretary of Labor may require the contractor to set aside in a separate account assets for the meeting of obligations under the plan or program.

2. Withholding

The contracting agency shall upon its own action or upon written request of an authorized representative of the Department of Labor, withhold or cause to be withheld from the contractor under this contract, or any other Federal contract with the same prime contractor, or any other federallyassisted contract subject to Davis-Bacon prevailing wage requirements, which is held by the same prime contractor, so much of the accrued payments or advances as may be considered necessary to pay laborers and mechanics, including apprentices, trainees, and helpers, employed by the contractor or any subcontractor the full amount of wages required by the contract. In the event of failure to pay any laborer or mechanic, including any apprentice, trainee, or helper, employed or working on the site of the work, all or part of the wages required by the contract, the contracting agency may, after written notice to the contractor, take such action as may be necessary to cause the suspension of any further payment, advance, or guarantee of funds until such violations have ceased.

3. Payrolls and basic records

a. Payrolls and basic records relating thereto shall be maintained by the contractor during the course of the work and preserved for a period of three years thereafter for all laborers and mechanics working at the site of the work. Such records shall contain the name, address, and social security number of each such worker, his or her correct classification, hourly rates of wages paid (including rates of contributions or costs anticipated for bona fide fringe benefits or cash equivalents thereof of the types described in section 1(b)(2)(B) of the Davis-Bacon Act), daily and weekly number of hours worked, deductions made and actual wages paid. Whenever the Secretary of Labor has found under 29 CFR 5.5(a)(1)(iv) that the wages of any laborer or mechanic include the amount of any costs reasonably anticipated in providing benefits under a plan or program described in section 1(b)(2)(B) of the Davis-

Bacon Act, the contractor shall maintain records which show that the commitment to provide such benefits is enforceable, that the plan or program is financially responsible, and that the plan or program has been communicated in writing to the laborers or mechanics affected, and records which show the costs anticipated or the actual cost incurred in providing such benefits. Contractors employing apprentices or trainees under approved programs shall maintain written evidence of the registration of apprenticeship programs and certification of trainee programs, the registration of the apprentices and trainees, and the ratios and wage rates prescribed in the applicable programs.

- b.(1) The contractor shall submit weekly for each week in which any contract work is performed a copy of all payrolls to the contracting agency. The payrolls submitted shall set out accurately and completely all of the information required to be maintained under 29 CFR 5.5(a)(3)(i), except that full social security numbers and home addresses shall not be included on weekly transmittals. Instead the payrolls shall only need to include an individually identifying number for each employee (e.g., the last four digits of the employee's social security number). The required weekly payroll information may be submitted in any form desired. Optional Form WH-347 is available for this purpose from the Wage and Hour Division Web site at http://www.dol.gov/esa/whd/forms/wh347instr.htm or its successor site. The prime contractor is responsible for the submission of copies of payrolls by all subcontractors Contractors and subcontractors shall maintain the full social security number and current address of each covered worker, and shall provide them upon request to the contracting agency for transmission to the State DOT, the FHWA or the Wage and Hour Division of the Department of Labor for purposes of an investigation or audit of compliance with prevailing wage requirements. It is not a violation of this section for a prime contractor to require a subcontractor to provide addresses and social security numbers to the prime contractor for its own records, without weekly submission to the contracting agency..
- (2) Each payroll submitted shall be accompanied by a "Statement of Compliance," signed by the contractor or subcontractor or his or her agent who pays or supervises the payment of the persons employed under the contract and shall certify the following:
 - (i) That the payroll for the payroll period contains the information required to be provided under §5.5 (a)(3)(ii) of Regulations, 29 CFR part 5, the appropriate information is being maintained under §5.5 (a)(3)(i) of Regulations, 29 CFR part 5, and that such information is correct and complete;
 - (ii) That each laborer or mechanic (including each helper, apprentice, and trainee) employed on the contract during the payroll period has been paid the full weekly wages earned, without rebate, either directly or indirectly, and that no deductions have been made either directly or indirectly from the full wages earned, other than permissible deductions as set forth in Regulations, 29 CFR part 3;
 - (iii) That each laborer or mechanic has been paid not less than the applicable wage rates and fringe benefits or cash equivalents for the classification of work performed, as specified in the applicable wage determination incorporated into the contract.

- (3) The weekly submission of a properly executed certification set forth on the reverse side of Optional Form WH–347 shall satisfy the requirement for submission of the "Statement of Compliance" required by paragraph 3.b.(2) of this section.
- (4) The falsification of any of the above certifications may subject the contractor or subcontractor to civil or criminal prosecution under section 1001 of title 18 and section 231 of title 31 of the United States Code.
- c. The contractor or subcontractor shall make the records required under paragraph 3.a. of this section available for inspection, copying, or transcription by authorized representatives of the contracting agency, the State DOT, the FHWA, or the Department of Labor, and shall permit such representatives to interview employees during working hours on the job. If the contractor or subcontractor fails to submit the required records or to make them available, the FHWA may, after written notice to the contractor, the contracting agency or the State DOT, take such action as may be necessary to cause the suspension of any further payment, advance, or guarantee of funds. Furthermore, failure to submit the required records upon request or to make such records available may be grounds for debarment action pursuant to 29 CFR 5.12.

4. Apprentices and trainees

a. Apprentices (programs of the USDOL).

Apprentices will be permitted to work at less than the predetermined rate for the work they performed when they are employed pursuant to and individually registered in a bona fide apprenticeship program registered with the U.S. Department of Labor, Employment and Training Administration, Office of Apprenticeship Training, Employer and Labor Services, or with a State Apprenticeship Agency recognized by the Office, or if a person is employed in his or her first 90 days of probationary employment as an apprentice in such an apprenticeship program, who is not individually registered in the program, but who has been certified by the Office of Apprenticeship Training, Employer and Labor Services or a State Apprenticeship Agency (where appropriate) to be eligible for probationary employment as an apprentice.

The allowable ratio of apprentices to journeymen on the job site in any craft classification shall not be greater than the ratio permitted to the contractor as to the entire work force under the registered program. Any worker listed on a payroll at an apprentice wage rate, who is not registered or otherwise employed as stated above, shall be paid not less than the applicable wage rate on the wage determination for the classification of work actually performed. In addition, any apprentice performing work on the job site in excess of the ratio permitted under the registered program shall be paid not less than the applicable wage rate on the wage determination for the work actually performed. Where a contractor is performing construction on a project in a locality other than that in which its program is registered, the ratios and wage rates (expressed in percentages of the journeyman's hourly rate) specified in the contractor's or subcontractor's registered program shall be observed.

Every apprentice must be paid at not less than the rate specified in the registered program for the apprentice's level of progress, expressed as a percentage of the journeymen hourly

rate specified in the applicable wage determination. Apprentices shall be paid fringe benefits in accordance with the provisions of the apprenticeship program. If the apprenticeship program does not specify fringe benefits, apprentices must be paid the full amount of fringe benefits listed on the wage determination for the applicable classification. If the Administrator determines that a different practice prevails for the applicable apprentice classification, fringes shall be paid in accordance with that determination.

In the event the Office of Apprenticeship Training, Employer and Labor Services, or a State Apprenticeship Agency recognized by the Office, withdraws approval of an apprenticeship program, the contractor will no longer be permitted to utilize apprentices at less than the applicable predetermined rate for the work performed until an acceptable program is approved.

b. Trainees (programs of the USDOL).

Except as provided in 29 CFR 5.16, trainees will not be permitted to work at less than the predetermined rate for the work performed unless they are employed pursuant to and individually registered in a program which has received prior approval, evidenced by formal certification by the U.S. Department of Labor, Employment and Training Administration.

The ratio of trainees to journeymen on the job site shall not be greater than permitted under the plan approved by the Employment and Training Administration.

Every trainee must be paid at not less than the rate specified in the approved program for the trainee's level of progress, expressed as a percentage of the journeyman hourly rate specified in the applicable wage determination. Trainees shall be paid fringe benefits in accordance with the provisions of the trainee program. If the trainee program does not mention fringe benefits, trainees shall be paid the full amount of fringe benefits listed on the wage determination unless the Administrator of the Wage and Hour Division determines that there is an apprenticeship program associated with the corresponding journeyman wage rate on the wage determination which provides for less than full fringe benefits for apprentices. Any employee listed on the payroll at a trainee rate who is not registered and participating in a training plan approved by the Employment and Training Administration shall be paid not less than the applicable wage rate on the wage determination for the classification of work actually performed. In addition, any trainee performing work on the job site in excess of the ratio permitted under the registered program shall be paid not less than the applicable wage rate on the wage determination for the work actually performed.

In the event the Employment and Training Administration withdraws approval of a training program, the contractor will no longer be permitted to utilize trainees at less than the applicable predetermined rate for the work performed until an acceptable program is approved.

c. Equal employment opportunity. The utilization of apprentices, trainees and journeymen under this part shall be in conformity with the equal employment opportunity requirements of Executive Order 11246, as amended, and 29 CFR part 30. d. Apprentices and Trainees (programs of the U.S. DOT).

Apprentices and trainees working under apprenticeship and skill training programs which have been certified by the Secretary of Transportation as promoting EEO in connection with Federal-aid highway construction programs are not subject to the requirements of paragraph 4 of this Section IV. The straight time hourly wage rates for apprentices and trainees under such programs will be established by the particular programs. The ratio of apprentices and trainees to journeymen shall not be greater than permitted by the terms of the particular program.

- 5. Compliance with Copeland Act requirements. The contractor shall comply with the requirements of 29 CFR part 3, which are incorporated by reference in this contract.
- 6. Subcontracts. The contractor or subcontractor shall insert Form FHWA-1273 in any subcontracts and also require the subcontractors to include Form FHWA-1273 in any lower tier subcontracts. The prime contractor shall be responsible for the compliance by any subcontractor or lower tier subcontractor with all the contract clauses in 29 CFR 5.5.
- 7. Contract termination: debarment. A breach of the contract clauses in 29 CFR 5.5 may be grounds for termination of the contract, and for debarment as a contractor and a subcontractor as provided in 29 CFR 5.12.
- Compliance with Davis-Bacon and Related Act requirements. All rulings and interpretations of the Davis-Bacon and Related Acts contained in 29 CFR parts 1, 3, and 5 are herein incorporated by reference in this contract.
- 9. Disputes concerning labor standards. Disputes arising out of the labor standards provisions of this contract shall not be subject to the general disputes clause of this contract. Such disputes shall be resolved in accordance with the procedures of the Department of Labor set forth in 29 CFR parts 5, 6, and 7. Disputes within the meaning of this clause include disputes between the contractor (or any of its subcontractors) and the contracting agency, the U.S. Department of Labor, or the employees or their representatives.

10. Certification of eligibility.

- a. By entering into this contract, the contractor certifies that neither it (nor he or she) nor any person or firm who has an interest in the contractor's firm is a person or firm ineligible to be awarded Government contracts by virtue of section 3(a) of the Davis-Bacon Act or 29 CFR 5.12(a)(1).
- b. No part of this contract shall be subcontracted to any person or firm ineligible for award of a Government contract by virtue of section 3(a) of the Davis-Bacon Act or 29 CFR 5.12(a)(1).
- c. The penalty for making false statements is prescribed in the U.S. Criminal Code, 18 U.S.C. 1001.

V. CONTRACT WORK HOURS AND SAFETY STANDARDS ACT

The following clauses apply to any Federal-aid construction contract in an amount in excess of \$100,000 and subject to the overtime provisions of the Contract Work Hours and Safety Standards Act. These clauses shall be inserted in addition to the clauses required by 29 CFR 5.5(a) or 29 CFR 4.6. As used in this paragraph, the terms laborers and mechanics include watchmen and quards.

- 1. Overtime requirements. No contractor or subcontractor contracting for any part of the contract work which may require or involve the employment of laborers or mechanics shall require or permit any such laborer or mechanic in any workweek in which he or she is employed on such work to work in excess of forty hours in such workweek unless such laborer or mechanic receives compensation at a rate not less than one and one-half times the basic rate of pay for all hours worked in excess of forty hours in such workweek.
- 2. Violation; liability for unpaid wages; liquidated damages. In the event of any violation of the clause set forth in paragraph (1.) of this section, the contractor and any subcontractor responsible therefor shall be liable for the unpaid wages. In addition, such contractor and subcontractor shall be liable to the United States (in the case of work done under contract for the District of Columbia or a territory, to such District or to such territory), for liquidated damages. Such liquidated damages shall be computed with respect to each individual laborer or mechanic, including watchmen and guards, employed in violation of the clause set forth in paragraph (1.) of this section, in the sum of \$10 for each calendar day on which such individual was required or permitted to work in excess of the standard workweek of forty hours without payment of the overtime wages required by the clause set forth in paragraph (1.) of this section.
- 3. Withholding for unpaid wages and liquidated damages. The FHWA or the contacting agency shall upon its own action or upon written request of an authorized representative of the Department of Labor withhold or cause to be withheld, from any moneys payable on account of work performed by the contractor or subcontractor under any such contract or any other Federal contract with the same prime contractor, or any other federally-assisted contract subject to the Contract Work Hours and Safety Standards Act, which is held by the same prime contractor, such sums as may be determined to be necessary to satisfy any liabilities of such contractor or subcontractor for unpaid wages and liquidated damages as provided in the clause set forth in paragraph (2.) of this section.
- 4. Subcontracts. The contractor or subcontractor shall insert in any subcontracts the clauses set forth in paragraph (1.) through (4.) of this section and also a clause requiring the subcontractors to include these clauses in any lower tier subcontracts. The prime contractor shall be responsible for compliance by any subcontractor or lower tier subcontractor with the clauses set forth in paragraphs (1.) through (4.) of this section.

VI. SUBLETTING OR ASSIGNING THE CONTRACT

This provision is applicable to all Federal-aid construction contracts on the National Highway System.

- 1. The contractor shall perform with its own organization contract work amounting to not less than 30 percent (or a greater percentage if specified elsewhere in the contract) of the total original contract price, excluding any specialty items designated by the contracting agency. Specialty items may be performed by subcontract and the amount of any such specialty items performed may be deducted from the total original contract price before computing the amount of work required to be performed by the contractor's own organization (23 CFR 635.116).
- a. The term "perform work with its own organization" refers to workers employed or leased by the prime contractor, and equipment owned or rented by the prime contractor, with or without operators. Such term does not include employees or equipment of a subcontractor or lower tier subcontractor, agents of the prime contractor, or any other assignees. The term may include payments for the costs of hiring leased employees from an employee leasing firm meeting all relevant Federal and State regulatory requirements. Leased employees may only be included in this term if the prime contractor meets all of the following conditions:
- (1) the prime contractor maintains control over the supervision of the day-to-day activities of the leased employees;
 - (2) the prime contractor remains responsible for the quality of the work of the leased employees;
- (3) the prime contractor retains all power to accept or exclude individual employees from work on the project; and
 - (4) the prime contractor remains ultimately responsible for the payment of predetermined minimum wages, the submission of payrolls, statements of compliance and all other Federal regulatory requirements.
- b. "Specialty Items" shall be construed to be limited to work that requires highly specialized knowledge, abilities, or equipment not ordinarily available in the type of contracting organizations qualified and expected to bid or propose on the contract as a whole and in general are to be limited to minor components of the overall contract.
- The contract amount upon which the requirements set forth in paragraph (1) of Section VI is computed includes the cost of material and manufactured products which are to be purchased or produced by the contractor under the contract provisions.
- 3. The contractor shall furnish (a) a competent superintendent or supervisor who is employed by the firm, has full authority to direct performance of the work in accordance with the contract requirements, and is in charge of all construction operations (regardless of who performs the work) and (b) such other of its own organizational resources (supervision, management, and engineering services) as the contracting officer determines is necessary to assure the performance of the contract.
- 4. No portion of the contract shall be sublet, assigned or otherwise disposed of except with the written consent of the contracting officer, or authorized representative, and such consent when given shall not be construed to relieve the contractor of any responsibility for the fulfillment of the contract. Written consent will be given only after the contracting agency has assured that each subcontract is

evidenced in writing and that it contains all pertinent provisions and requirements of the prime contract.

 The 30% self-performance requirement of paragraph (1) is not applicable to design-build contracts; however, contracting agencies may establish their own self-performance requirements.

VII. SAFETY: ACCIDENT PREVENTION

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts.

- 1. In the performance of this contract the contractor shall comply with all applicable Federal, State, and local laws governing safety, health, and sanitation (23 CFR 635). The contractor shall provide all safeguards, safety devices and protective equipment and take any other needed actions as it determines, or as the contracting officer may determine, to be reasonably necessary to protect the life and health of employees on the job and the safety of the public and to protect property in connection with the performance of the work covered by the contract.
- 2. It is a condition of this contract, and shall be made a condition of each subcontract, which the contractor enters into pursuant to this contract, that the contractor and any subcontractor shall not permit any employee, in performance of the contract, to work in surroundings or under conditions which are unsanitary, hazardous or dangerous to his/her health or safety, as determined under construction safety and health standards (29 CFR 1926) promulgated by the Secretary of Labor, in accordance with Section 107 of the Contract Work Hours and Safety Standards Act (40 U.S.C. 3704).
- 3. Pursuant to 29 CFR 1926.3, it is a condition of this contract that the Secretary of Labor or authorized representative thereof, shall have right of entry to any site of contract performance to inspect or investigate the matter of compliance with the construction safety and health standards and to carry out the duties of the Secretary under Section 107 of the Contract Work Hours and Safety Standards Act (40 U.S.C.3704).

VIII. FALSE STATEMENTS CONCERNING HIGHWAY PROJECTS

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts.

In order to assure high quality and durable construction in conformity with approved plans and specifications and a high degree of reliability on statements and representations made by engineers, contractors, suppliers, and workers on Federal-aid highway projects, it is essential that all persons concerned with the project perform their functions as carefully, thoroughly, and honestly as possible. Willful falsification, distortion, or misrepresentation with respect to any facts related to the project is a violation of Federal law. To prevent any misunderstanding regarding the seriousness of these and similar acts, Form FHWA-1022 shall be posted on each Federal-aid highway project (23 CFR 635) in one or more places where it is readily available to all persons concerned with the project:

18 U.S.C. 1020 reads as follows:

"Whoever, being an officer, agent, or employee of the United States, or of any State or Territory, or whoever, whether a person, association, firm, or corporation, knowingly makes any false statement, false representation, or false report as to the character, quality, quantity, or cost of the material used or to be used, or the quantity or quality of the work performed or to be performed, or the cost thereof in connection with the submission of plans, maps, specifications, contracts, or costs of construction on any highway or related project submitted for approval to the Secretary of Transportation; or

Whoever knowingly makes any false statement, false representation, false report or false claim with respect to the character, quality, quantity, or cost of any work performed or to be performed, or materials furnished or to be furnished, in connection with the construction of any highway or related project approved by the Secretary of Transportation; or

Whoever knowingly makes any false statement or false representation as to material fact in any statement, certificate, or report submitted pursuant to provisions of the Federal-aid Roads Act approved July 1, 1916, (39 Stat. 355), as amended and supplemented;

Shall be fined under this title or imprisoned not more than 5 years or both."

IX. IMPLEMENTATION OF CLEAN AIR ACT AND FEDERAL WATER POLLUTION CONTROL ACT

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts.

By submission of this bid/proposal or the execution of this contract, or subcontract, as appropriate, the bidder, proposer, Federal-aid construction contractor, or subcontractor, as appropriate, will be deemed to have stipulated as follows:

- 1. That any person who is or will be utilized in the performance of this contract is not prohibited from receiving an award due to a violation of Section 508 of the Clean Water Act or Section 306 of the Clean Air Act.
- 2. That the contractor agrees to include or cause to be included the requirements of paragraph (1) of this Section X in every subcontract, and further agrees to take such action as the contracting agency may direct as a means of enforcing such requirements.

X. CERTIFICATION REGARDING DEBARMENT, SUSPENSION, INELIGIBILITY AND VOLUNTARY EXCLUSION

This provision is applicable to all Federal-aid construction contracts, design-build contracts, subcontracts, lower-tier subcontracts, purchase orders, lease agreements, consultant contracts or any other covered transaction requiring FHWA approval or that is estimated to cost \$25,000 or more – as defined in 2 CFR Parts 180 and 1200.

1. Instructions for Certification - First Tier Participants:

- a. By signing and submitting this proposal, the prospective first tier participant is providing the certification set out below.
- b. The inability of a person to provide the certification set out below will not necessarily result in denial of participation in this

covered transaction. The prospective first tier participant shall submit an explanation of why it cannot provide the certification set out below. The certification or explanation will be considered in connection with the department or agency's determination whether to enter into this transaction. However, failure of the prospective first tier participant to furnish a certification or an explanation shall disqualify such a person from participation in this transaction.

- c. The certification in this clause is a material representation of fact upon which reliance was placed when the contracting agency determined to enter into this transaction. If it is later determined that the prospective participant knowingly rendered an erroneous certification, in addition to other remedies available to the Federal Government, the contracting agency may terminate this transaction for cause of default.
- d. The prospective first tier participant shall provide immediate written notice to the contracting agency to whom this proposal is submitted if any time the prospective first tier participant learns that its certification was erroneous when submitted or has become erroneous by reason of changed circumstances.
- e. The terms "covered transaction," "debarred,"
 "suspended," "ineligible," "participant," "person," "principal,"
 and "voluntarily excluded," as used in this clause, are defined
 in 2 CFR Parts 180 and 1200. "First Tier Covered
 Transactions" refers to any covered transaction between a
 grantee or subgrantee of Federal funds and a participant (such
 as the prime or general contract). "Lower Tier Covered
 Transactions" refers to any covered transaction under a First
 Tier Covered Transaction (such as subcontracts). "First Tier
 Participant" refers to the participant who has entered into a
 covered transaction with a grantee or subgrantee of Federal
 funds (such as the prime or general contractor). "Lower Tier
 Participant" refers any participant who has entered into a
 covered transaction with a First Tier Participant or other Lower
 Tier Participants (such as subcontractors and suppliers).
- f. The prospective first tier participant agrees by submitting this proposal that, should the proposed covered transaction be entered into, it shall not knowingly enter into any lower tier covered transaction with a person who is debarred, suspended, declared ineligible, or voluntarily excluded from participation in this covered transaction, unless authorized by the department or agency entering into this transaction.
- g. The prospective first tier participant further agrees by submitting this proposal that it will include the clause titled "Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion-Lower Tier Covered Transactions," provided by the department or contracting agency, entering into this covered transaction, without modification, in all lower tier covered transactions and in all solicitations for lower tier covered transactions exceeding the \$25,000 threshold.
- h. A participant in a covered transaction may rely upon a certification of a prospective participant in a lower tier covered transaction that is not debarred, suspended, ineligible, or voluntarily excluded from the covered transaction, unless it knows that the certification is erroneous. A participant is responsible for ensuring that its principals are not suspended, debarred, or otherwise ineligible to participate in covered transactions. To verify the eligibility of its principals, as well as the eligibility of any lower tier prospective participants, each participant may, but is not required to, check the Excluded Parties List System website (https://www.epls.gov/), which is compiled by the General Services Administration.

- i. Nothing contained in the foregoing shall be construed to require the establishment of a system of records in order to render in good faith the certification required by this clause. The knowledge and information of the prospective participant is not required to exceed that which is normally possessed by a prudent person in the ordinary course of business dealings.
- j. Except for transactions authorized under paragraph (f) of these instructions, if a participant in a covered transaction knowingly enters into a lower tier covered transaction with a person who is suspended, debarred, ineligible, or voluntarily excluded from participation in this transaction, in addition to other remedies available to the Federal Government, the department or agency may terminate this transaction for cause or default.

2. Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion – First Tier Participants:

- a. The prospective first tier participant certifies to the best of its knowledge and belief, that it and its principals:
- Are not presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from participating in covered transactions by any Federal department or agency;
- (2) Have not within a three-year period preceding this proposal been convicted of or had a civil judgment rendered against them for commission of fraud or a criminal offense in connection with obtaining, attempting to obtain, or performing a public (Federal, State or local) transaction or contract under a public transaction; violation of Federal or State antitrust statutes or commission of embezzlement, theft, forgery, bribery, falsification or destruction of records, making false statements, or receiving stolen property;
- (3) Are not presently indicted for or otherwise criminally or civilly charged by a governmental entity (Federal, State or local) with commission of any of the offenses enumerated in paragraph (a)(2) of this certification; and
- (4) Have not within a three-year period preceding this application/proposal had one or more public transactions (Federal, State or local) terminated for cause or default.
- b. Where the prospective participant is unable to certify to any of the statements in this certification, such prospective participant shall attach an explanation to this proposal.

2. Instructions for Certification - Lower Tier Participants:

(Applicable to all subcontracts, purchase orders and other lower tier transactions requiring prior FHWA approval or estimated to cost \$25,000 or more - 2 CFR Parts 180 and 1200)

- a. By signing and submitting this proposal, the prospective lower tier is providing the certification set out below.
- b. The certification in this clause is a material representation of fact upon which reliance was placed when this transaction was entered into. If it is later determined that the prospective lower tier participant knowingly rendered an erroneous certification, in addition to other remedies available to the Federal Government, the department, or agency with which

this transaction originated may pursue available remedies, including suspension and/or debarment.

- c. The prospective lower tier participant shall provide immediate written notice to the person to which this proposal is submitted if at any time the prospective lower tier participant learns that its certification was erroneous by reason of changed circumstances.
- d. The terms "covered transaction," "debarred,"
 "suspended," "ineligible," "participant," "person," "principal,"
 and "voluntarily excluded," as used in this clause, are defined
 in 2 CFR Parts 180 and 1200. You may contact the person to
 which this proposal is submitted for assistance in obtaining a
 copy of those regulations. "First Tier Covered Transactions"
 refers to any covered transaction between a grantee or
 subgrantee of Federal funds and a participant (such as the
 prime or general contract). "Lower Tier Covered Transactions"
 refers to any covered transaction under a First Tier Covered
 Transaction (such as subcontracts). "First Tier Participant"
 refers to the participant who has entered into a covered
 transaction with a grantee or subgrantee of Federal funds
 (such as the prime or general contractor). "Lower Tier
 Participant" refers any participant who has entered into a
 covered transaction with a First Tier Participant or other Lower
 Tier Participants (such as subcontractors and suppliers).
- e. The prospective lower tier participant agrees by submitting this proposal that, should the proposed covered transaction be entered into, it shall not knowingly enter into any lower tier covered transaction with a person who is debarred, suspended, declared ineligible, or voluntarily excluded from participation in this covered transaction, unless authorized by the department or agency with which this transaction originated.
- f. The prospective lower tier participant further agrees by submitting this proposal that it will include this clause titled "Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion-Lower Tier Covered Transaction," without modification, in all lower tier covered transactions and in all solicitations for lower tier covered transactions exceeding the \$25,000 threshold.
- g. A participant in a covered transaction may rely upon a certification of a prospective participant in a lower tier covered transaction that is not debarred, suspended, ineligible, or voluntarily excluded from the covered transaction, unless it knows that the certification is erroneous. A participant is responsible for ensuring that its principals are not suspended, debarred, or otherwise ineligible to participate in covered transactions. To verify the eligibility of its principals, as well as the eligibility of any lower tier prospective participants, each participant may, but is not required to, check the Excluded Parties List System website (https://www.epls.gov/), which is compiled by the General Services Administration.
- h. Nothing contained in the foregoing shall be construed to require establishment of a system of records in order to render in good faith the certification required by this clause. The knowledge and information of participant is not required to exceed that which is normally possessed by a prudent person in the ordinary course of business dealings.
- i. Except for transactions authorized under paragraph e of these instructions, if a participant in a covered transaction knowingly enters into a lower tier covered transaction with a person who is suspended, debarred, ineligible, or voluntarily excluded from participation in this transaction, in addition to other remedies available to the Federal Government, the

department or agency with which this transaction originated may pursue available remedies, including suspension and/or debarment.

Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion--Lower Tier Participants:

- The prospective lower tier participant certifies, by submission of this proposal, that neither it nor its principals is presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from participating in covered transactions by any Federal department or agency.
- Where the prospective lower tier participant is unable to certify to any of the statements in this certification, such prospective participant shall attach an explanation to this proposal.

XI. CERTIFICATION REGARDING USE OF CONTRACT FUNDS FOR LOBBYING

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts which exceed \$100,000 (49 CFR 20).

- 1. The prospective participant certifies, by signing and submitting this bid or proposal, to the best of his or her knowledge and belief, that:
- a. No Federal appropriated funds have been paid or will be paid, by or on behalf of the undersigned, to any person for influencing or attempting to influence an officer or employee of any Federal agency, a Member of Congress, an officer or employee of Congress, or an employee of a Member of Congress in connection with the awarding of any Federal contract, the making of any Federal grant, the making of any Federal loan, the entering into of any cooperative agreement, and the extension, continuation, renewal, amendment, or modification of any Federal contract, grant, loan, or cooperative agreement.
- b. If any funds other than Federal appropriated funds have been paid or will be paid to any person for influencing or attempting to influence an officer or employee of any Federal agency, a Member of Congress, an officer or employee of Congress, or an employee of a Member of Congress in connection with this Federal contract, grant, loan, or cooperative agreement, the undersigned shall complete and submit Standard Form-LLL, "Disclosure Form to Report Lobbying," in accordance with its instructions.
- 2. This certification is a material representation of fact upon which reliance was placed when this transaction was made or entered into. Submission of this certification is a prerequisite for making or entering into this transaction imposed by 31 U.S.C. 1352. Any person who fails to file the required certification shall be subject to a civil penalty of not less than \$10,000 and not more than \$100,000 for each such failure.
- 3. The prospective participant also agrees by submitting its bid or proposal that the participant shall require that the language of this certification be included in all lower tier subcontracts, which exceed \$100,000 and that all such recipients shall certify and disclose accordingly.

8 PROSECUTION AND PROGRESS

10-19-12

Replace "working days" in the 1st paragraph of section 8-1.02B(1) with:

original working days

10-19-12

Replace "working days" at each occurrence in the 1st paragraph of section 8-1.02C(1) with:

original working days

10-19-12

Delete the 4th paragraph of section 8-1.02C(1).

04-20-12

Replace "Contract" in the 9th paragraph of section 8-1.02C(1) with:

work

10-19-12

Replace the 1st paragraph of section 8-1.02C(3)(a) with:

Submit a description of your proposed schedule software for authorization.

04-20-12

Delete the last paragraph of section 8-1.02C(3)(a).

04-20-12

Replace section 8-1.02C(3)(b) with:

8-1.02C(3)(b) Reserved

10-19-12

Delete the 3rd paragraph of section 8-1.02C(5).

04-20-12

Replace "Contract" in the last paragraph of section 8-1.02C(5) with:

original

10-19-12

Replace "working days" in the 1st paragraph of section 8-1.02D(1) with:

original working days

10-19-12

Replace "8-1.02D(1)" in the 2nd paragraph of section 8-1.02D(1) with:

01-20-12

8-1.02C(1)

Replace "Contract" in the 3rd paragraph of section 8-1.02D(2) with:

10-19-12 work

Replace "Contract" in item 9 in the list in the 4th paragraph of section 8-1.02D(4) with:

10-19-12 work

Replace "Contract completion" in the 4th paragraph of section 8-1.02D(6) with:

work completion

Replace "Contract working days" in the 4th paragraph of section 8-1.02D(6) with:

original working days

04-20-12

Delete items 1.3 and 1.4 in the list in the 1st paragraph of section 8-1.02D(10).

Replace the last paragraph of section 8-1.04B with:

10-19-12

The Department does not adjust time for starting before receiving notice of Contract approval.

Replace the 1st paragraph of section 8-1.05 with:

10-19-12

Contract time starts on the last day specified to start job site activities in section 8-1.04 or on the day you start job site activities, whichever occurs first.

Replace the 2nd paragraph of section 8-1.05 with:

10-19-12

Complete the work within the Contract time.

10-19-12

Delete "unless the Contract is suspended for reasons unrelated to your performance" in the 4th paragraph of section 8-1.05.

Replace the headings and paragraphs in section 8-1.06 with:

10-19-12

The Engineer may suspend work wholly or in part due to conditions unsuitable for work progress. Provide for public safety and a smooth and unobstructed passageway through the work zone during the suspension as specified under sections 7-1.03 and 7-1.04. Providing the passageway is force account work. The Department makes a time adjustment for the suspension due to a critical delay.

The Engineer may suspend work wholly or in part due to your failure to (1) fulfill the Engineer's orders, (2) fulfill a Contract part, or (3) perform weather-dependent work when conditions are favorable so that weather-related unsuitable conditions are avoided or do not occur. The Department may provide for a

smooth and unobstructed passageway through the work during the suspension and deduct the cost from payments. The Department does not make a time adjustment for the suspension.

Upon the Engineer's order of suspension, suspend work immediately. Resume work when ordered.

Replace the 1st sentence in the 1st paragraph of section 8-1.07B with:

10-19-12

For a critical delay, the Department may make a time adjustment.

Add to the end of section 8-1.07C:

10-19-12

The Department does not make a payment adjustment for overhead incurred during non–working days that extend the Contract into an additional construction season.

Replace the 1st paragraph of section 8-1.07C with:

10-19-12

For an excusable delay that affects your costs, the Department may make a payment adjustment.

Replace "8-1.08B and 8-1.08C" in the 1st paragraph of section 8-1.10A with:

08-05-11

8-1.10B and 8-1.10C

Replace section 8-1.10D with:

10-19-12

8-1.10D Reserved

9 PAYMENT

01-18-13

Replace item 1 in the 3rd paragraph of section 9-1.03 with:

01-18-13

 Full compensation for all work involved in each bid item shown on the Bid Item List by the unit of measure shown for that bid item

Replace "in" in the 3rd paragraph of section 9-1.04A with:

10-19-12

for

Add to the end of section 9-1.04A:

10-19-12

For nonsubcontracted work paid by force account for a contract with a TRO bid item, the markups are those shown in the following table instead of those specified in sections 9-1.04B–D:

Cost	Percent markup		
Labor	30		
Materials	10		
Equipment rental	10		

04-20-12

Delete ", Huntington Beach," in the 3rd paragraph of section 9-1.07A.

Replace the formula in section 9-1.07B(2) with:

04-20-12

 $Qh = HMATT \times Xa$

Replace "weight of dry aggregate" in the definition of the variable Xa in section 9-1.07B(2) with:

total weight of HMA

04-20-12

Replace the formula in section 9-1.07B(3) with:

04-20-12

 $Qrh = RHMATT \times 0.80 \times Xarb$

Replace "weight of dry aggregate" in the definition of the variable Xarb in section 9-1.07B(3) with:

04-20-12

total weight of rubberized HMA

Replace the heading of section 9-1.07B(4) with:

04-20-12

Hot Mix Asphalt with Modified Asphalt Binder

Add between "in" and "modified" in the introductory clause of section 9-1.07B(4):

04-20-12

HMA with

Replace the formula in section 9-1.07B(4) with:

04-20-12

 $Qmh = MHMATT \times [(100 - Xam) / 100] \times Xmab$

Replace "weight of dry aggregate" in the definition of the variable Xmab in section 9-1.07B(4) with:

04-20-12

total weight of HMA

Replace the formula in section 9-1.07B(5) with:

04-20-12

Qrap = HMATT x Xaa

Replace "weight of dry aggregate" in the definitions of the variables *Xaa* and *Xta* in section 9-1.07B(5) with:

total weight of HMA

Add after the variable definitions in section 9-1.07B(9):

04-20-12

The quantity of extender oil is included in the quantity of asphalt.

Replace the headings and paragraphs in section 9-1.11 with:

10-19-12

9-1.11A General

Section 9-1.11 applies if a bid item for time-related overhead is included in the Contract. If a bid item for time-related overhead is included, you must exclude the time-related overhead from every other bid item price.

9-1.11B Payment Quantity

The TRO quantity does not include the number of working days to complete plant establishment work.

For a contract with a TRO lump sum quantity on the Bid Item List, the Department pays you based on the following conversions:

- 1. LS unit of measure is replaced with WDAY
- 2. Lump sum quantity is replaced with the number of working days bid
- 3. Lump sum unit price is replaced with the item total divided by the number of working days bid

9-1.11C Payment Inclusions

Payment for the TRO bid item includes payment for time-related field- and home-office overhead for the time required to complete the work.

The field office overhead includes time-related expenses associated with the normal and recurring construction activities not directly attributed to the work, including:

- 1. Salaries, benefits, and equipment costs of:
 - 1.1. Project managers
 - 1.2. General superintendents
 - 1.3. Field office managers
 - 1.4. Field office staff assigned to the project
- 2. Rent
- 3. Utilities
- 4. Maintenance
- 5. Security
- 6. Supplies
- 7. Office equipment costs for the project's field office

The home-office overhead includes the fixed general and administrative expenses for operating your business, including:

- 1. General administration
- 2. Insurance
- 3. Personnel and subcontract administration
- 4. Purchasing
- 5. Accounting
- 6. Project engineering and estimating

Payment for the TRO bid item does not include payment for:

- 1. The home-office overhead expenses specifically related to:
 - 1.1. Your other contracts or other businesses
 - 1.2. Equipment coordination
 - 1.3. Material deliveries
 - 1.4. Consultant and legal fees
- 2. Non-time-related costs and expenses such as mobilization, licenses, permits, and other charges incurred once during the Contract
- 3. Additional overhead involved in incentive/disincentive provisions to satisfy an internal milestone or multiple calendar requirements
- 4. Additional overhead involved in performing additional work that is not a controlling activity
- 5. Overhead costs incurred by your subcontractors of any tier or suppliers

9-1.11D Payment Schedule

For progress payments, the total work completed for the TRO bid item is the number of working days shown for the pay period on the *Weekly Statement of Working Days*.

For progress payments, the Department pays a unit price equal to the lesser of the following amounts:

- 1. Price per working day as bid or as converted under section 9-1.11B.
- 2. 20 percent of the total bid divided by the number of original working days

For a contract without plant establishment work, the Department pays you the balance due of the TRO item total as specified in section 9-1.17B.

For a contract with plant establishment work, the Department pays you the balance due of the TRO item total in the 1st progress payment after all non–plant establishment work is completed.

9-1.11E Payment Adjustments

The 3rd paragraph of section 9-1.17C does not apply.

The Department does not adjust the unit price for an increase or decrease in the TRO quantity except as specified in section 9-1.11E.

Section 9-1.17D(2)(b) does not apply except as specified for the audit report below.

If the TRO bid item quantity exceeds 149 percent of the quantity shown on the Bid Item List or as converted under section 9-1.11B, the Engineer may adjust or you may request an adjustment of the unit price for the excess quantity. For the adjustment, submit an audit report within 60 days of the Engineer's request. The report must be prepared as specified for an audit report for an overhead claim in section 9-1.17D(2)(b).

Within 20 days of the Engineer's request, make your financial records available for an audit by the State for the purpose of verifying the actual rate of TRO described in your audit. The actual rate of TRO described is subject to the Engineer's authorization.

The Department pays the authorized actual rate for TRO in excess of 149 percent of the quantity shown on the Bid Item List or as converted under section 9-1.11B.

The Department pays for 1/2 the cost of the report; the Contractor pays for the other 1/2. The cost is determined under section 9-1.05.

10-19-12

Delete "revised Contract" in item 1 of the 1st paragraph of section 9-1.16E(2).

Replace "2014" in the 1st paragraph of section 9-1.16F with:

10-19-12

2020

Replace the 2nd paragraph of section 9-1.17C with:

10-19-12

Submit either a written acceptance of the proposed final estimate or a claim statement postmarked or hand delivered before the 31st day after receiving the proposed final estimate.

Add between "the" and "final estimate" in the 1st sentence in the 3rd paragraph of section 9-1.17C:

proposed 10-19-12

^^^^^

DIVISION II GENERAL CONSTRUCTION 10 GENERAL

07-20-12

Replace "Reserved" in section 10-1 with:

01-20-12

10-1.01 GENERAL

Reserved

10-1.02 WORK SEQUENCING

Before obliterating any traffic stripes, pavement markings, and pavement markers to be replaced at the same location, reference the stripes, markings, and markers. Include limits and transitions with control points to reestablish the new stripes, markings, and markers.

10-1.03 TIME CONSTRAINTS

Reserved

10-1.04-10-1.10 RESERVED

Replace "Reserved" in section 10-2.01 with:

07-20-12

10-2.01A General

CALGREEN TIER 1

Reserved

10-2.01B-10-2.01H Reserved

Replace the heading of section 10-2.02 with:

•

Replace section 10-2.03 with:

07-20-12

07-20-12

10-2.03 LEED

10-2.03A-10-2.03H Reserved

12 TEMPORARY TRAFFIC CONTROL

10-19-12

Replace the 1st paragraph of section 12-3.01A(4) with:

10-19-12

Category 2 temporary traffic control devices must be on FHWA's list of acceptable, crashworthy Category 2 hardware for work zones. This list is available on FHWA's Safety Program Web site.

Replace "project" in the 4th paragraph of section 12-3.02C with:

10-19-12

work

Replace "project" in the 3rd paragraph of section 12-3.07C with:

10-19-12

work

Add between the 7th and 8th paragraphs of section 12-4.03:

10-19-12

The contingency plan must identify the operations, equipment, processes, and materials that may fail and delay a reopening of a closure to traffic. List the additional or alternate equipment, materials, or workers necessary to ensure continuing operations and on-time opening of closures whenever a problem occurs. If the additional or alternate equipment, materials, or workers are not on site, specify their location, the method for mobilizing these items, and the required time to complete mobilization.

Based on the Engineer's review, additional materials, equipment, workers, or time to complete operations from that specified in the contingency plan may be required.

Provide a general time-scaled logic diagram displaying the major activities and sequence of planned operations that comply with the requirements of section 12-4.03. For each operation, identify the critical event when the contingency plan will be activated.

Submit any revisions to the contingency plan for an operation at least 3 business days before starting that operation. Do not close any lanes until the contingency plan has been authorized.

The 5th paragraph of section 5-1.23B(1) does not apply to reviewing contingency plans.

Replace section 12-7 with:

09-16-11

12-7 RESERVED

13 WATER POLLUTION CONTROL

10-19-12 Add to section 13-1.01A:

01-20-12

Comply with the Department's general permit issued by the State Water Resources Control Board for Order No. 99-06-DWQ, NPDES No. CAS000003, National Pollutant Discharge Elimination System (NPDES) Permit, Statewide Storm Water Permit and Waste Discharge Requirements (WDRs) for the

State of California, Department of Transportation (Caltrans). The Department's general permit governs stormwater and nonstormwater discharges from the Department's properties, facilities, and activities. The Department's general permit may be viewed at the Web site for the State Water Resources Control Board, Storm Water Program, Caltrans General Permit.

Add to the list in the 1st paragraph of section 13-1.01D(3)(b): 10-21-11 3. Have completed SWRCB approved QSD training and passed the QSD exam Add to the list in the 2nd paragraph of section 13-1.01D(3)(b): 10-21-11 3. Have completed SWRCB approved QSP training and passed the QSP exam Replace "working days" at each occurrence in section 13-3.04 with. 10-19-12 original working days Replace the paragraph in section 13-4.04 with: 04-20-12 Not Used 10-19-12 Delete "or stockpile" in the 3rd paragraph of section 13-5.02F. Replace section 13-5.03F with: 04-20-12 13-5.03F Reserved 10-19-12 Delete "or stockpile" in item 1 in the list in the 1st paragraph of section 13-5.03K. 10-19-12 Delete the 3rd paragraph of section 13-5.03K. Replace the 2nd sentence in the 1st paragraph of section 13-9.01A with: 10-19-12 You may use any of the following systems for temporary concrete washout: 1. Temporary concrete washout facility 2. Portable temporary concrete washout 3. Temporary concrete washout bin

Retain and submit an informational submittal for records of disposed concrete waste.

Replace the 2nd paragraph of section 13-9.01B with:

10-19-12

Delete the 4th paragraph of section 13-9.01B.

10-19-12

Delete "if authorized" in the 1st sentence in the 1st paragraph of section 13-9.02A.

Replace "at least 3-inch" in the 3rd sentence in the 1st paragraph of section 13-9.02A with:

10-19-12

6-inch

^^^^^^

15 EXISTING FACILITIES

01-18-13

Replace the 4th paragraph of section 15-2.10B with:

01-18-13

Instead of using new materials similar in character to those in the existing structure, you may use raising devices to adjust a manhole to grade. Before starting paving work, measure and fabricate raising devices. Raising devices must:

- 1. Comply with the specifications for section 75 except that galvanizing is not required
- 2 Have a shape and size that matches the existing frame
- 3. Be match marked by painting identification numbers on the device and corresponding structure
- 4. Result in an installation that is equal to or better than the existing one in stability, support, and nonrocking characteristics
- 5. Be fastened securely to the existing frame without projections above the surface of the road or into the clear opening

Replace the 1st paragraph of section 15-5.01C(1) with:

10-19-12

Before starting deck rehabilitation activities, complete the removal of any traffic stripes, pavement markings, and pavement markers.

Replace the 2nd and 3rd paragraphs of section 15-5.01C(2) with:

10-19-12

Perform the following activities in the order listed:

- 1. Abrasive blast the deck surface with steel shot. Perform abrasive blasting after the removal of any unsound concrete and placement of any rapid setting concrete patches.
- 2. Sweep the deck surface.
- 3. Blow the deck surface clean using high-pressure air.

Replace the 2nd paragraph of section 15-5.01C(4) with:

10-19-12

Before removing asphalt concrete surfacing, verify the depth of the surfacing at the supports and midspans of each structure (1) in each shoulder, (2) in the traveled way, and (3) at the roadway crown, if a crown is present.

Replace the 2nd paragraph of section 15-5.03A(2) with:

10-19-12

For a contract with less than 60 original working days, submit certificates of compliance for the filler material and bonding agents.

Replace the 4th paragraph of section 15-5.03B with:

10-19-12

For a contract with less than 60 original working days, alternative materials must be authorized before use.

Add between the 5th and 6th paragraphs of section 15-5.03C:

The final surface finish of the patched concrete surface must comply with section 51-1.03F.

10-19-12

10-19-12

Delete the 4th paragraph of section 15-5.05C.

Replace "51-1.03F(5)" in the 3rd paragraph of section 15-5.06C(1) with:

51-1.01D(4)

10-19-12

Replace "51-1.03E(5)" in the 5th paragraph of section 15-5.06C(1) with:

51-1.03F(5)

10-19-12

Delete the 9th paragraph of section 15-5.06C(1).

10-19-12

Add to section 15-5.06C(1):

Texture the polyester concrete surface before gelling occurs by longitudinal tining under 51-

1.03F(5)(b)(iii), except do not perform initial texturing.

10-19-12

Replace the 1st paragraph in section 15-5.07B(4) with:

10-19-12

Payment for furnishing dowels is not included in the payment for core and pressure grout dowel.

Replace the heading of section 15-6.04 with:

INVERT PAVING

01-18-13

^^^^^^

DIVISION III GRADING 19 EARTHWORK

01-18-13

Replace the 2nd paragraph of section 19-3.01A(2)(b) with:

07-01-11

For cofferdams on or affecting railroad property, allow 85 days for review.

Add to the list in the 1st paragraph of section 19-3.01A(2)(d):

01-20-12

9. Provisions for discontinuous rows of soil nails

Add to section 19-3.01A(3)(b):

01-20-12

For soil nail walls, wall zones are specified in the special provisions.

For ground anchor walls, a wall zone is the entire wall unless otherwise specified in the special provisions.

01-20-12

Delete the 2nd sentence in the 4th paragraph of section 19-3.01A(3)(b).

Replace "90" in the paragraph of section 19-3.02G with:

01-18-13

90-1

Replace the 1st paragraph of section 19-3.03E(3) with:

01-20-12

Compact structure backfill behind lagging of soldier pile walls by hand tamping, mechanical compaction, or other authorized means.

Replace the 2nd paragraph of section 19-3.03F with:

01-20-12

Do not backfill over or place material over slurry cement backfill until 4 hours after placement. When concrete sand is used as aggregate and the in-place material is free draining, you may start backfilling as soon as the surface water is gone.

Add between the 2nd and 3rd paragraphs of section 19-3.03K:

01-20-12

Before you excavate for the installation of ground anchors in a wall zone:

- 1. Complete stability testing
- 2. Obtain authorization of test data

Replace the 2nd sentence of the 7th paragraph of section 19-3.03K:

01-20-12

Stop construction in unstable areas until remedial measures have been taken. Remedial measures must be submitted and authorized.

Add between the 8th and 9th paragraphs of section 19-3.03K:

01-20-12

When your excavation and installation methods result in a discontinuous wall along any soil nail row, the ends of the structurally completed wall section must extend beyond the ends of the next lower excavation lift by a distance equal to twice the lift height. Maintain temporary slopes at the ends of each wall section to ensure slope stability.

Replace the 9th paragraph of section 19-3.03K:

01-20-12

Do not excavate to the next underlying excavation lift until the following conditions have been attained for the portion of the soil nail or ground anchor wall in the current excavation lift:

- 1. Soil nails or ground anchors are installed and grouted.
- 2. Reinforced shotcrete facing is constructed.

01-18-13

3. Grout and shotcrete have cured for at least 72 hours.

01-20-12

- 4. Specified tests are complete for that portion of wall and the results are authorized.
- 5. Soil nail facing anchorages are attached or ground anchors are locked off.

Replace the 2nd sentence in the 7th paragraph of section 19-3.04 with:

01-18-13

Structure excavation more than 0.5 foot from the depth shown is paid for as a work-character change if you request an adjustment or the Engineer orders an adjustment.

Replace "Contract completion time" in the 8th paragraph of section 19-6.03D with:

10-19-12

work completion date

Add to section 19:

01-18-13

19-10-19-20 RESERVED

^^^^^^

20 LANDSCAPE

10-19-12

10-19-12

Add "preparing holes," before "and" in the 1st paragraph of section 20-7.01A.

Replace "and handling" in the 1st paragraph of section 20-7.03A with:

handling, and preparing holes

10-19-12

Replace the 1st paragraph of section 20-7.03D with:

10-19-12

The location of all plants is as shown unless the Engineer designates otherwise. If the Engineer designates the location of plants, the location will be marked by stakes, flags, or other markers.

Replace item 1 in the list in the 1st paragraph of section 20-7.03l with:

1. Preparing holes and planting plants

10-19-12

10-19-12

Delete "Prepare Hole," in the last paragraph of section 20-7.04.

^^^^^^

21 EROSION CONTROL

01-18-13

Replace ", bonded fiber matrix, and polymer-stabilized fiber matrix" in the 1st paragraph of section 21-1.01B with:

and bonded fiber matrix

04-20-12

Delete the last paragraph of section 21-1.02E.

04-20-12

Replace section 21-1.02F(2) with:

21-1.02F(2) Reserved

04-20-12

Replace section 21-1.02J with:

04-20-12

21-1.02J Reserved

Replace the row for organic matter content in the table in the 4th paragraph of section 21-1.02M with:

			<u>01-1</u> 8-13
Organic matter	TMECC 05.07-A	30–100	
content	Loss-on-ignition organic matter method (LOI)		
	% dry weight basis		

Replace the paragraph in section 21-1.02P with:

10-19-12

Fiber roll must be a premanufactured roll filled with rice or wheat straw, wood excelsior, or coconut fiber. Fiber roll must be covered with biodegradable jute, sisal, or coir fiber netting secured tightly at each end and must be one of the following:

- 1. 8 to 10 inches in diameter and at least 1.1 lb/ft
- 2. 10 to 12 inches in diameter and at least 3 lb/ft

Fiber roll must have a minimum functional longevity of 1 year.

Add between the 1st and 2nd paragraphs of section 21-1.03A:

01-18-13

Remove and dispose of trash, debris, and weeds in areas to receive erosion control materials.

Remove and dispose of loose rocks larger than 2-1/2 inches in maximum dimension unless otherwise authorized.

Protect the traveled way, sidewalks, lined drainage channels, and existing vegetation from overspray of hydraulically-applied material.

Replace section 21-1.03B with:

01-18-13

21-1.03B Reserved

Replace section 21-1.03I with:

04-20-12

21-1.03I Reserved

Add between the 4th and 5th paragraphs of section 21-1.03P:

10-19-12

If soil conditions do not permit driving the stakes into the soil, drill pilot holes to facilitate driving of the stakes.

01-18-13

Delete the 1st and 2nd sentences of the 3rd paragraph in section 21-1.04.

DIVISION IV SUBBASES AND BASES 29 TREATED PERMEABLE BASES

04-20-12

Replace "section 68-4.02C" in the 6th paragraph of section 29-1.03A with:

04-20-12

section 64-4.03

^^^^^^

Replace section 30 with:

04-20-12

30 RECLAIMED PAVEMENTS

04-20-12 **30-1 GENERAL**

30-1.01 GENERAL

Section 30 includes specifications for reclaiming the pavement section and constructing a base.

30-2 FULL DEPTH RECLAIMED—FOAMED ASPHALT

Reserved

30-3-30-6 RESERVED

^^^^^^

DIVISION V SURFACINGS AND PAVEMENTS 37 BITUMINOUS SEALS

01-18-13 **Replace section 37-1.01 with:**

01-18-13

37-1.01 GENERAL

37-1.01A Summary

Section 37-1 includes general specifications for applying bituminous seals.

37-1.01B Definitions

Reserved

37-1.01C Submittals

Reserved

37-1.01D Quality Control and Assurance

37-1.01D(1) General

Reserved

37-1.01D(2) Prepaying Conference

For seal coats and micro-surfacing, schedule a prepaving conference at a mutually agreed upon time and place to meet with the Engineer.

Prepaving conference attendees must sign an attendance sheet provided by the Engineer. The prepaving conference must be attended by your:

- 1. Project superintendent
- 2. Paving construction foreman
- 3. Traffic control foreman

Be prepared to discuss:

- 1. Quality control
- 2. Acceptance testing
- 3. Placement
- 4. Training on placement methods
- 5. Checklist of items for proper placement
- 6. Unique issues specific to the project, including:
 - 6.1. Weather
 - 6.2. Alignment and geometrics

- 6.3. Traffic control issues
- 6.4. Haul distances
- 6.5. Presence and absence of shaded areas
- 6.6. Any other local issues

37-1.02 MATERIALS

Not Used

37-1.03 CONSTRUCTION

Not Used

37-1.04 PAYMENT

Not Used

Replace "Reserved" in section 37-2.01D(1) with:

01-18-13

Aggregate suppliers, chip spreader operators, emulsion distributor, and for coated chips, the coated chips producer must attend the prepaving conference.

Add to section 37-2.03A:

04-20-12

If you fail to place the permanent traffic stripes and pavement markings within the specified time, the Department withholds 50 percent of the estimated value of the seal coat work completed that has not received permanent traffic stripes and pavement markings.

Add to section 37-3.01D(1):

01-18-13

Micro-surfacing spreader operators must attend the prepaving conference.

39 HOT MIX ASPHALT

02-22-13 Add to section 39-1.01B:

02-22-13

processed RAP: RAP that has been fractionated.

substitution rate: Amount of RAP aggregate substituted for virgin aggregate in percent.

binder replacement: Amount of RAP binder in OBC in percent.

surface course: Upper 0.2 feet of HMA exclusive of OGFC.

Add to the end of the paragraph in section 39-1.02A:

10-19-12

as shown

Replace the paragraphs in section 39-1.02F with:

02-22-13

39-1.02F(1) General

You may produce HMA Type A or B using RAP. HMA produced using RAP must comply with the specifications for HMA, except aggregate quality specifications do not apply to RAP. You may substitute RAP at a substitution rate not exceeding 25 percent of the aggregate blend. Do not use RAP in OGFC and RHMA-G.

Assign the substitution rate of RAP aggregate for virgin aggregate with the JMF submittal. The JMF must include the percent of RAP used.

Provide enough space for meeting RAP handling requirements at your facility. Provide a clean, graded, well-drained area for stockpiles. Prevent material contamination and segregation.

If RAP is from multiple sources, blend the RAP thoroughly and completely. RAP stockpiles must be homogeneous.

Isolate the processed RAP stockpiles from other materials. Store processed RAP in conical or longitudinal stockpiles. Processed RAP must not be agglomerated or be allowed to congeal in large stockpiles.

AASHTO T 324 (Modified) is AASHTO T 324, "Hamburg Wheel-Track Testing of Compacted Hot Mix Asphalt (HMA)," with the following parameters:

- 1. Target air voids must equal 7 ± 1 percent
- 2. Number of test specimens must be 4
- 3. Test specimen must be a 6-inch gyratory compacted specimen
- 4. Test temperature must be set at 140 ± 2 degrees F
- 5. Measurements for impression must be taken at every 100 passes
- 6. Inflection point defined as the number of wheel passes at the intersection of the creep slope and the stripping slope
- 7. Testing shut off must be set at 25,000 passes

39-1.02F(2) Substitution Rate of 15 Percent or Less

For a RAP substitution rate of 15 percent or less, you may stockpile RAP during the entire project.

39-1.02F(3) Substitution Rate Greater than 15 Percent

For a RAP substitution rate greater than 15 percent, fractionate RAP into 2 sizes, a coarse fraction RAP retained on 1/4-inch screen and a fine fraction RAP passing 1/4-inch screen.

Sample and test processed RAP at a minimum frequency of 1 sample per 1000 tons with a minimum of 6 samples for each processed RAP stockpile. The asphalt binder content and specific gravity must meet the processed RAP quality characteristics. If a processed RAP stockpile is augmented, sample and test processed RAP quality characteristics at a minimum frequency of 1 sample per 500 tons of augmented RAP.

The processed RAP asphalt binder content must be within \pm 2.0 percent of the average processed RAP stockpile asphalt binder content when tested under ASTM D 2172, Method B. If a new processed RAP stockpile is required, the average binder content of the new processed RAP stockpile must be within \pm 2.0 percent of the average binder content of the original processed RAP stockpile.

The maximum specific gravity for processed RAP must be within ± 0.06 when tested under California Test 309 of the average maximum specific gravity reported on page 4 of your *Contractor Hot Mix Asphalt Design Data* form.

Replace "less than 10 percent" in note "b" in the table in the 5th paragraph of section 39-1.02E with:

01-20-12

10 percent or less

Replace items 7 and 8 in the 5th paragraph of section 39-1.03A with:

02-22-13

- 7. Substitution rate by more than 5 percent if your assigned RAP substitution rate is 15 percent or less
- 8. Substitution rate by more than 3 percent if your assigned RAP substitution rate is greater than 15 percent
- 9. Average binder content by more than 2 percent from the average binder content of the original processed RAP stockpile used in the mix design
- 10. Maximum specific gravity of processed RAP by more than ±0.060 from the average maximum specific gravity of processed RAP reported on page 4 of your *Contractor Hot Mix Asphalt Design Data* form
- 11. Any material in the JMF

Replace the 1st paragraph of section 39-1.03B with:

02-22-13

Perform a mix design that produces HMA with the values for the quality characteristics shown in the following table:

HMA Mix Design Requirements

Quality characteristic	Test	HMA type			
-	method	Α	В	RHMA-G	
Air void content (%)	California	4.0	4.0	Section 39-1.03B	
	Test 367				
Voids in mineral aggregate (% min.)	California				
No. 4 grading	Test 367	17.0	17.0		
3/8" grading		15.0	15.0		
1/2" grading		14.0	14.0	18.0–23.0	
3/4" grading		13.0	13.0	18.0–23.0	
Voids filled with asphalt (%)	California			Note a	
No. 4 grading	Test 367	65.0-75.0	65.0–75.0		
3/8" grading		65.0–75.0	65.0–75.0		
1/2" grading		65.0–75.0	65.0–75.0		
3/4" grading		65.0-75.0	65.0–75.0		
Dust proportion	California			Note a	
No. 4 and 3/8" gradings	Test 367	0.6-1.2	0.6–1.2		
1/2" and 3/4" gradings		0.6-1.2	0.6–1.2		
Stabilometer value (min.)	California				
No. 4 and 3/8" gradings	Test 366	30	30		
1/2" and 3/4" gradings		37	35	23	

^a Report this value in the JMF submittal.

For RAP substitution rate greater than 15 percent, the mix design must comply with the additional quality characteristics shown in the following table:

Additional HMA Mix Design Requirements for RAP Substitution Rate Greater Than 15 Percent

Quality characteristic	Test method	HMA type			
		Α	В	RHMA-G	
Hamburg wheel track	AASHTO				
(minimum number of passes at 0.5	T 324				
inch average rut depth)	(Modified) ^a				
PG-58		10,000	10,000		
PG-64		15,000	15,000		
PG-70		20,000	20,000		
PG-76 or higher		25,000	25,000		
Hamburg wheel track	AASHTO				
(inflection point minimum number of	T 324				
passes) [†]	(Modified) ^a				
PG-58		10,000	10,000		
PG-64		10,000	10,000		
PG-70		12,500	12,500		
PG-76 or higher		15000	15000		
Moisture susceptibility	California	120	120		
(minimum dry strength, psi)	Test 371 ^a	120	120		
Moisture susceptibility	California	70	70		
(tensile strength ration, %)	Test 371 ^a	70	70		

^aTest plant produced HMA.

For HMA with RAP, the maximum binder replacement must be 25.0 percent of OBC for surface course and 40.0 percent of OBC for lower courses.

For HMA with a binder replacement less than or equal to 25 percent of OBC, you may request that the PG asphalt binder grade with upper and lower temperature classifications be reduced by 6 degrees C from the specified grade.

For HMA with a binder replacement greater than 25 percent but less than or equal to 40 percent of OBC, you must use a PG asphalt binder grade with upper and lower temperature classifications reduced by 6 degrees C from the specified grade.

Replace item 4 in the list in the 1st paragraph of section 39-1.03C with:

4. JMF renewal on a Caltrans Job Mix Formula Renewal form, if applicable

01-20-12

Add after the last paragraph of section 39-1.03C:

02-22-13

For RAP substitution rate greater than 15 percent, submit with the JMF submittal:

- 1. California Test 371 tensile strength ratio and minimum dry strength test results
- 2. AASHTO T 324 (Modified) test results

For RAP substitution rate greater than 15 percent, submit California Test 371 and AASHTO T 324 (Modified) test results to the Engineer and to:

Moisture_Tests@dot.ca.gov

Replace the 2nd paragraph of section 39-1.03E with:

04-20-12

Use the OBC specified on your *Contractor Hot Mix Asphalt Design Data* form. No adjustments to asphalt binder content are allowed. Based on your testing and production experience, you may submit an adjusted aggregate gradation TV on a *Contractor Job Mix Formula Proposal* form before verification testing. Aggregate gradation TV must be within the TV limits specified in the aggregate gradation tables.

Add between the 3rd and 4th paragraphs of section 39-1.03E:

04-20-12

Asphalt binder set point for HMA must be the OBC specified on your *Contractor Hot Mix Asphalt Design Data* form. When RAP is used, asphalt binder set point for HMA must be:

Asphalt Binder Set Point =
$$\frac{\frac{BC_{OBC}}{\left(1 - \frac{BC_{OBC}}{100}\right)} - R_{RAP} \left[\frac{BC_{RAP}}{\left(1 - \frac{BC_{RAP}}{100}\right)}\right]}{\frac{BC_{OBC}}{\left(1 - \frac{BC_{OBC}}{100}\right)}}$$

Where:

BC_{OBC} = optimum asphalt binder content, percent based on total weight of mix

 $R_{RAP} = RAP$ ratio by weight of aggregate

BC_{RAP} = asphalt binder content of RAP, percent based on total weight of RAP mix

Replace item 4 in the list in the 8th paragraph of section 39-1.03E with:

04-20-12

- 4. HMA quality specified in the table titled "HMA Mix Design Requirements" except:
 - 4.1. Air void content, design value ±2.0 percent
 - 4.2. Voids filled with asphalt, report only
 - 4.3. Dust proportion, report only

Replace the 12th paragraph of section 39-1.03E with:

04-20-12

If tests on plant-produced samples do not verify the JMF, the Engineer notifies you and you must submit a new JMF or submit an adjusted JMF based on your testing. JMF adjustments may include a change in aggregate gradation TV within the TV limits specified in the aggregate gradation tables.

Replace the 14th paragraph of section 39-1.03E with:

01-20-12

A verified JMF is valid for 12 months.

Replace the last sentence in the 15th paragraph of section 39-1.03E with:

01-20-12

This deduction does not apply to verifications initiated by the Engineer or JMF renewal.

Replace the 16th paragraph of section 39-1.03E with:

02-22-13

Except for RAP substitution rate greater than 15 percent, for any HMA produced under the QC/QA process the Department does not use California Test 371 test results for verification.

Add between the 1st and 2nd paragraphs of section 39-1.03F:

04-20-12

Target asphalt binder content on your Contractor *Job Mix Formula Proposal* form and the OBC specified on your *Contractor Hot Mix Asphalt Design Data* form must be the same.

01-20-12

Delete the 4th paragraph of section 39-1.03F.

Replace items 3 and 5 in the list in the 6th paragraph of section 39-1.03F with:

01-20-12

- 3. Engineer verifies each proposed JMF renewal within 20 days of receiving verification samples.
- 5. For each HMA type and aggregate gradation specified, the Engineer verifies at the Department's expense 1 proposed JMF renewal within a 12-month period.

Add between the 6th and 7th paragraphs of section 39-1.03F:

01-20-12

The most recent aggregate quality test results within the past 12 months may be used for verification of JMF renewal or the Engineer may perform aggregate quality tests for verification of JMF renewal.

Replace section 39-1.03G with:

04-20-12

39-1.03G Job Mix Formula Modification

For an accepted JMF, you may change asphalt binder source one time during production.

Submit your modified JMF request a minimum of 3 business days before production. Each modified JMF submittal must consist of:

- 1. Proposed modified JMF on Contractor Job Mix Formula Proposal form
- Mix design records on Contractor Hot Mix Asphalt Design Data form for the accepted JMF to be modified
- 3. JMF verification on Hot Mix Asphalt Verification form for the accepted JMF to be modified
- 4. Quality characteristics test results for the modified JMF as specified in section 39-1.03B. Perform tests at the mix design OBC as shown on the *Contractor Asphalt Mix Design Data* form
- 5. If required, California Test 371 test results for the modified JMF.

With an accepted modified JMF submittal, the Engineer verifies each modified JMF within 5 business days of receiving all verification samples. If California Test 371 is required, the Engineer tests for California Test 371 within 10 days of receiving verification samples.

The Engineer verifies the modified JMF after the modified JMF HMA is placed on the project and verification samples are taken within the first 750 tons following sampling requirements in section 39-1.03E, "Job Mix Formula Verification." The Engineer tests verification samples for compliance with:

- 1. Stability as shown in the table titled "HMA Mix Design Requirements"
- 2. Air void content at design value ±2.0 percent
- 3. Voids in mineral aggregate as shown in the table titled "HMA Mix Design Requirements"
- 4. Voids filled with asphalt, report only

5. Dust proportion, report only

If the modified JMF is verified, the Engineer revises your *Hot Mix Asphalt Verification* form to include the new asphalt binder source. Your revised form will have the same expiration date as the original form.

If a modified JMF is not verified, stop production and any HMA placed using the modified JMF is rejected.

The Engineer deducts \$2,000 from payments for each modified JMF verification. The Engineer deducts an additional \$2,000 for each modified JMF verification that requires California Test 371.

Add to section 39-1.03:

01-20-12

39-1.03H Job Mix Formula Acceptance

You may start HMA production if:

- 1. The Engineer's review of the JMF shows compliance with the specifications.
- 2. The Department has verified the JMF within 12 months before HMA production.
- 3. The Engineer accepts the verified JMF.

Replace "3 days" in the 1st paragraph of section 39-1.04A with:

3 business days

01-20-12

Replace the 2nd sentence in the 2nd paragraph of section 39-1.04A with:

01-20-12

During production, take samples under California Test 125. You may sample HMA from:

Replace the 2nd paragraph of section 39-1.04E with:

02-22-13

For RAP substitution rate of 15 percent or less, sample RAP once daily.

For RAP substitution rate of greater than 15percent, sample processed RAP twice daily.

Perform QC testing for processed RAP aggregate gradation under California Test 367, appendix B, and submit the results with the combined aggregate gradation.

Replace "5 days" in the 1st paragraph of section 39-1.06 with:

01-20-12

5 business days

Replace the 3rd paragraph of section 39-1.08A with:

04-20-12

During production, you may adjust hot or cold feed proportion controls for virgin aggregate and RAP.

Add to section 39-1.08A:

04-20-12

During production, asphalt binder set point for HMA Type A, HMA Type B, HMA Type C, and RHMA-G must be the OBC shown in *Contractor Hot Mix Asphalt Design Data* form. For OGFC, asphalt binder set

point must be the OBC shown on *Caltrans Hot Mix Asphalt Verification* form. If RAP is used, asphalt binder set point for HMA must be calculated as specified in section 39-1.03E.

02-22-13

For RAP substitution rate of 15 percent or less, you may adjust the RAP by ±5 percent.

For RAP substitution greater than 15, you may adjust the RAP by ±3 percent.

04-20-12

You must request adjustments to the plant asphalt binder set point based on new RAP stockpiles average asphalt binder content. Do not adjust the HMA plant asphalt binder set point until authorized.

Replace the 3rd paragraph of section 39-1.08B with:

09-16-11

Asphalt rubber binder must be from 375 to 425 degrees F when mixed with aggregate.

Replace section 39-1.11 with:

01-18-13

39-1.11 CONSTRUCTION

39-1.11A General

Do not place HMA on wet pavement or a frozen surface.

You may deposit HMA in a windrow and load it in the paver if:

- 1. Paver is equipped with a hopper that automatically feeds the screed
- 2. Loading equipment can pick up the windrowed material and deposit it in the paver hopper without damaging base material
- 3. Activities for deposit, pickup, loading, and paving are continuous
- 4. HMA temperature in the windrow does not fall below 260 degrees F

You may place HMA in 1 or more layers on areas less than 5 feet wide and outside the traveled way, including shoulders. You may use mechanical equipment other than a paver for these areas. The equipment must produce uniform smoothness and texture.

HMA handled, spread, or windrowed must not stain the finished surface of any improvement, including pavement.

Do not use petroleum products such as kerosene or diesel fuel to release HMA from trucks, spreaders, or compactors.

HMA must be free of:

- Segregation
- 2. Coarse or fine aggregate pockets
- 3. Hardened lumps

39-1.11B Longitudinal Joints

39-1.11B(1) General

Longitudinal joints in the top layer must match specified lane edges. Alternate the longitudinal joint offsets in the lower layers at least 0.5 foot from each side of the specified lane edges. You may request other longitudinal joint placement patterns.

A vertical longitudinal joint of more than 0.15 ft is not allowed at any time between adjacent lanes open to traffic.

For HMA thickness of 0.15 ft or less, the distance between the ends of the adjacent surfaced lanes at the end of each day's work must not be greater than can be completed in the following day of normal paving.

For HMA thickness greater than 0.15 ft, you must place HMA on adjacent traveled way lanes so that at the end of each work shift the distance between the ends of HMA layers on adjacent lanes is from 5 to 10 feet. Place additional HMA along the transverse edge at each lane's end and along the exposed longitudinal edges between adjacent lanes. Hand rake and compact the additional HMA to form temporary conforms. You may place Kraft paper or another authorized bond breaker under the conform tapers to facilitate the taper removal when paving operations resume.

39-1.11B(2) Tapered Notched Wedge

For divided highways with an HMA lift thickness greater than 0.15 foot, you may construct a 1-foot wide tapered notched wedge joint as a longitudinal joint between adjacent lanes open to traffic. A vertical notch of 0.75 inch maximum must be placed at the top and bottom of the tapered wedge.

The tapered notched wedge must retain its shape while exposed to traffic. Pave the adjacent lane within 1 day.

Construct the tapered portion of the tapered notched wedge with an authorized strike-off device. The strike-off device must provide a uniform slope and must not restrict the main screed of the paver.

You may use a device attached to the screed to construct longitudinal joints that will form a tapered notched wedge in a single pass. The tapered notched wedge must be compacted to a minimum of 91 percent compaction.

Perform QC testing on the completed tapered notch wedge joint as follows:

- 1. Perform field compaction tests at the rate of 1 test for each 750-foot section along the joint. Select random locations for testing within each 750-foot section.
- 2. Perform field compaction tests at the centerline of the joint, 6 inches from the upper vertical notch, after the adjacent lane is placed and before opening the pavement to traffic.
- 3. Determine maximum density test results.
- 4. Determine percent compaction of the longitudinal joint as the ratio of the average of the field compaction values and the maximum density test results.

For HMA under QC/QA construction process, the additional quality control compaction results associated with the tapered notch wedge will not be included in the computation of any quality factor and process control.

For acceptance of the completed tapered notch wedge joint, take two 4- or 6-inch diameter cores 6 inches from the upper vertical notch of the completed longitudinal joint for every 3,000 feet at locations designated by the Engineer. Take cores after the adjacent lane is placed and before opening the pavement to traffic. Cores must be taken in the presence of the Engineer and must be marked to identify the test sites. Submit the cores. One core will be used for determination of the field density and 1 core will be used for dispute resolution. The Engineer determines:

- Field compaction by measuring the bulk specific gravity of the cores under California Test 308, Method A
- 2. Percent compaction as the ratio of the average of the bulk specific gravity of the core for each day's production to the maximum density test value

For HMA under QC/QA construction process, the additional quality assurance testing by the Engineer to determine field compaction associated with the tapered notch wedge will not be included in the Engineer's verification testing and in the computation of any quality factor and process control.

Determine percent compaction values each day the joint is completed and submit values within 24 hours of testing. If the percent compaction of 1 day's production is less than 91 percent, that day's notched wedge joint is rejected. Discontinue placement of the tapered notched wedge and notify the Engineer of changes you will make to your construction process in order to meet the specifications.

For HMA under QC/QA construction process, quantities of HMA placed in the completed longitudinal joint will have a quality factor QF_{QC5} of 1.0.

39-1.11C Widening Existing Pavement

If widening existing pavement, construct new pavement structure to match the elevation of the existing pavement's edge before placing HMA over the existing pavement.

39-1.11D Shoulders, Medians, and Other Road Connections

Until the adjoining through lane's top layer has been paved, do not pave the top layer of:

- Shoulders
- 2. Tapers
- 3. Transitions
- 4. Road connections
- 5. Driveways
- 6. Curve widenings
- 7. Chain control lanes
- 8. Turnouts
- 9. Turn pockets

If the number of lanes changes, pave each through lane's top layer before paving a tapering lane's top layer. Simultaneous to paving a through lane's top layer, you may pave an adjoining area's top layer, including shoulders. Do not operate spreading equipment on any area's top layer until completing final compaction.

39-1.11E Leveling

If leveling with HMA is specified, fill and level irregularities and ruts with HMA before spreading HMA over the base, existing surfaces, or bridge decks. You may use mechanical equipment other than a paver for these areas. The equipment must produce uniform smoothness and texture. HMA used to change an existing surface's cross slope or profile is not paid for as HMA (leveling).

If placing HMA against the edge of existing pavement, sawcut or grind the pavement straight and vertical along the joint and remove extraneous material.

39-1.11F Compaction

Rolling must leave the completed surface compacted and smooth without tearing, cracking, or shoving. Complete finish rolling activities before the pavement surface temperature is:

- 1. Below 150 degrees F for HMA with unmodified binder
- 2. Below 140 degrees F for HMA with modified binder
- 3. Below 200 degrees F for RHMA-G

If a vibratory roller is used as a finish roller, turn the vibrator off.

Do not use a pneumatic-tired roller to compact RHMA-G.

For Standard and QC/QA construction processes, if 3/4-inch aggregate grading is specified, you may use a 1/2-inch aggregate grading if the specified total paved thickness is at least 0.15 foot and less than 0.20 foot thick.

Spread and compact HMA under sections 39-3.03 and 39-3.04 if any of the following applies:

- 1. Specified paved thickness is less than 0.15 foot.
- 2. Specified paved thickness is less than 0.20 foot and 3/4-inch aggregate grading is specified and used.
- 3. You spread and compact at:
 - 3.1. Asphalt concrete surfacing replacement areas
 - 3.2. Leveling courses
 - 3.3. Areas for which the Engineer determines conventional compaction and compaction measurement methods are impeded

Do not open new HMA pavement to public traffic until its mid-depth temperature is below 160 degrees F.

If you request and if authorized, you may cool HMA Type A and Type B with water when rolling activities are complete. Apply water under section 17-3.

Spread sand at a rate from 1 to 2 lb/sq yd on new RHMA-G, RHMA-O, and RHMA-O-HB pavement when finish rolling is complete. Sand must be free of clay or organic matter. Sand must comply with section 90-1.02C(4)(c). Keep traffic off the pavement until spreading sand is complete.

Replace the 5th and 6th paragraphs of section 39-1.12C with:

07-20-12

On tangents and horizontal curves with a centerline radius of curvature 2,000 feet or more, the Pl₀ must be at most 2.5 inches per 0.1-mile section.

On horizontal curves with a centerline radius of curvature between 1,000 feet and 2,000 feet including pavement within the superelevation transitions, the PI₀ must be at most 5 inches per 0.1-mile section.

Add to section 39-1.12:

01-20-12

39-1.12E Reserved

Add to section 39-1.14:

01-20-12

Prepare the area to receive HMA for miscellaneous areas and dikes, including any excavation and backfill as needed.

Replace "6.8" in item 3 in the list in the 4th paragraph of section 39-1.14 with:

04-20-12

6.4

Replace "6.0" in item 3 in the list in the 4th paragraph of section 39-1.14 with:

04-20-12

5.7

Replace "6.8" in the 1st paragraph of section 39-1.15B with:

04-20-12

6.4

Replace "6.0" in the 1st paragraph of section 39-1.15B with:

04-20-12

5.7

Replace the 1st paragraph of section 39-2.02B with:

02-22-13

Perform sampling and testing at the specified frequency for the quality characteristics shown in the following table:

Minimum Quality Control—Standard Construction Process

Coulity characteristic characteris		Minimum Quality Control—Standard Construction Process					
Aggregate gradution	Quality	Test	Minimum		HMA	type	
Aggregate California gradation" Test 202 Tolerance	characteristic	method					
Aggregate gradation				Α	В	RHMA-G	OGFC
Test 202 Tolerance Toler			frequency				
Sand equivalent (min)	Aggregate	California	1 per 750	JMF ±	JMF ±	JMF ±	
Sand equivalent (min)	gradation ^a	Test 202	tons and	Tolerance ^b	Tolerance ^b	Tolerance ^b	Tolerance ^b
Test 217	Sand equivalent	California	any	47	42		
Asphalt binder content (%) Test 379 or 382 California content (%, max) Test 226 for 370 Test 366 Stabilometer value (min) Test 366 Susiness day, (min.) Test 367 Percent of crushed particles coarse aggregate (%, min) Chairsing plants and batch mixing plants and batch mixing plants are solved afaces Fine aggregate (%, min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face loss Angeles Rattler (%, max) California rest 205 California re	(min) ^c	Test 217	remaining				
Test 379	Asphalt binder	California	part at the	JMF±0.40	JMF±0.40	JMF ± 0.40	JMF ± 0.40
HMA moisture California Test 226 or 370 or 370 Test 226 or 370 or 3		Test 379	end of the				
Content (%, max) Field compaction (% max. theoretical density) de	, ,	or 382	project				
Content (%, max) Field compaction (% max. theoretical density) and selection of the stable of the selection	HMA moisture	California	1 per 2,500	1.0	1.0	1.0	1.0
Field compaction (% max. theoretical density). and the second particles (% min). The second part	content (%, max)	Test 226	tons but				
Field compaction (% max. theoretical density). ** The reflection of the particle of the parti	, , ,	or 370	not less				
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(% max. theoretical density).6. Stabilometer value (min) ⁵ No. 4 and 3/8* gradings 1/2* and 3/4* gradings 1/4* gra			paving day				
theoretical density) ^{6,5} Stabilometer value (min) ^c No. 4 and 3/8" gradings 1/2" and 3/4" gradings 1/4" gradings 1/4" and 3/4" gradings 1/4"	Field compaction	QC plan	2 per	91–97	91–97	91–97	
Stabilometer Stabilometer Value (min)° No. 4 and 3/8" gradings 1/2" and 3/4" gradings 1/2" and 3/4" gradings No. 4 and 3/4" gradings 1/2" and 3/4" gradings No. 4 and 3/4" gradings	(% max.		business				
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value (min)° No. 4 and 3/8" gradings Test 366 tons or 2 per 5 business days, whichever is greater 30 30 Air void content (%)°.1 California Test 367 California Test 367 4 ± 2 TV ± 2 Aggregate moisture content at continuous mixing plants and RAP moisture content at continuous mixing plants and batch	density) ^{a,e}						
No. 4 and 3/8" gradings 1/2" and 3/4" gradings 1/2" and 3/4" signated particles continuous mixing plants and batch mixing plan							
gradings 1/2" and 3/4" gradings Air void content (%)c-1" Aggregate moisture content at continuous mixing plants and Batch mixing plants and Percent of crushed particles coarse aggregate (%, min) One fractured faces Fine aggregate (%, min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 California Test 211 business days, whichever adays, whichever is greater 4 ± 2		Test 366					
1/2" and 3/4" gradings				30	30		
Air void content (%)°-1° Test 367 Aggregate Moisture content at continuous mixing plants and RAP moisture content at continuous mixing plants and batch mixing plants and batc							
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Continuous mixing plants and RAP moisture content at continuous mixing plants and batch mixing plant							
Aggregate moisture content at continuous mixing plants and RAP moisture content at continuous mixing plants and batch mixing p	Air void content		is greater	4 ± 2	4 ± 2	$TV \pm 2$	
moisture content at continuous mixing plants and RAP moisture content at continuous mixing plants and batch mixing plants and							
at continuous mixing plants and RAP moisture content at continuous mixing plants and batch mixing plan							
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mixing plants ⁹ Percent of crushed particles coarse aggregate (%, min) One fractured faces Two fractured faces Fine aggregate (%, min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 California Test 205 90 25 90 75 12 12 12							
Percent of crushed particles coarse aggregate (%, min) One fractured face Two fractured faces Fine aggregate (%, min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 California Test 205 90 25 90 75 12 12 12							
crushed particles coarse aggregate (%, min) One fractured face Two fractured faces Fine aggregate (%, min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 Test 205 90 25 90 75 90 75 90 75 12 12 12		California					
coarse aggregate (%, min) One fractured face Two fractured faces Fine aggregate (%, min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 Paggregate (%, min) 90 25 90 75 As designated in the QC plan. At least once per project 70 20 70 90 90 12 12 12							
(%, min) One fractured face 90 25 90 Two fractured faces As 75 90 75 Fine aggregate (%, min) designated in the QC plan. At least once retained on no. 8 sieve.) plan. At least once per project 70 20 70 90 Los Angeles Rattler (%, max) Loss at 100 California Test 211 Test 211 12 12 12 12		1681 203					
One fractured face Two fractured faces Two fractured faces Fine aggregate (%, min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 Possible Project As designated in the QC plan. At least once per project 70 20 70 90 75 75 75 76 77 77 78 78 79 79 70 90 70 90 70 90 70 90 70 90 70 90 70 90 70 90							
face Two fractured faces Fine aggregate (%, min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 Tost 211 As As designated in the QC plan. At least once per project 70 20 70 90 75 75 75 75 75 76 77 78 79 70 70 70 70 70 70 70 70 70				gn	25		gn
Two fractured faces Fine aggregate (%, min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 Fine aggregate (designated in the QC plan. At least once per project 70 20 70 90 75 90 90 12 12 12					20		
faces Fine aggregate (%, min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 As designated in the QC plan. At least once per project 70 20 70 90 90 12 12 12				75		90	75
Fine aggregate (%, min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 designated in the QC plan. At least once per project 70 20 70 90 90 12 12 12			As	'			
(%, min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 in the QC plan. At least once per project 70 20 70 90 70 90 12 12 12							
(Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 plan. At least once per project 70 20 70 90 12 12 12 12							
4 sieve and retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 Reast once per project 70 20 70 90 12 12 12 12							
retained on no. 8 sieve.) One fractured face Los Angeles Rattler (%, max) Loss at 100 Per project 70 20 70 90 12 12 12			•				
no. 8 sieve.) One fractured face 70 20 70 90 Los Angeles Rattler (%, max) California Test 211 12 12 12							
One fractured face 70 20 70 90 Los Angeles Rattler (%, max) Loss at 100 California Test 211 12 12 12							
Los Angeles Rattler (%, max) Loss at 100 California Test 211 12 12 12				70	20	70	90
Rattler (%, max) Test 211	face						
Loss at 100 12 12 12	Los Angeles	California					
Loss at 100 12 12 12		Test 211					
rev.				12		12	12
	rev.						

1 1500	T	T	45		10	40
Loss at 500			45	50	40	40
rev. Flat and	California		Report only	Report only	Report only	Report only
elongated	Test 235		neport only	neport only	neport only	neport only
particles (%, max	1031 200					
by weight @ 5:1)						
Fine aggregate	California		45	45	45	
angularity (%,	Test 234					
min) ^h						
Voids filled with	California					
asphalt (%)	Test 367					
No. 4 grading			65.0-75.0	65.0-75.0		
3/8" grading			65.0-75.0	65.0-75.0	Report only	
1/2" grading			65.0-75.0	65.0-75.0		
3/4" grading			65.0-75.0	65.0-75.0		
Voids in mineral	California					
aggregate (%	Test 367					
min) ⁱ						
No. 4 grading			17.0	17.0		
3/8" grading			15.0	15.0		
1/2" grading			14.0	14.0	18.0–23.0	
3/4" grading			13.0	13.0	18.0–23.0	
Dust proportion	California					
No. 4 and 3/8"	Test 367		0.6-1.2	0.6-1.2		
gradings					Report only	
1/2" and 3/4"			0.6–1.2	0.6–1.2		
gradings						
Hamburg wheel	AASHTO					
track	T 324	1 per				
(minimum number	(Modified)	10,000				
of passes at 0.5		tons or 1				
inch average rut		per project				
depth) ¹ PG-58		whichever	10,000	10,000		
PG-64		is more	15,000	15,000		
PG-70			20,000	20,000		
PG-76 or higher			25,000	25,000		
Hamburg wheel	AASHTO		20,000	20,000		
track	T 324	1 per				
(inflection point	(Modified)	10,000				
minimum number	(meamea)	tons or 1				
of passes)		per project				
PG-58		whichever	10,000	10,000		
PG-64		is more	10,000	10,000		
PG-70			12,500	12,500		
PG-76 or higher			15000	15000		
Moisture	California	For RAP				
susceptibility	Test 371	≥15%				
(minimum dry		1 per				
strength, psi) ^j		10,000	120	120		
		tons or 1	120	120		
		per project				
		whichever				
	0 111	is greater				
Moisture	California	For RAP				
susceptibility	Test 371	≥15%	70	70		
(tensile strength		1 per	70	70		
ration, %) ^J		10,000				
		tons or 1				

		per project whichever is greater				
Smoothness	Section 39-1.12		12-foot straight- edge, must grind, and PI ₀			
Asphalt rubber binder viscosity @ 375 °F, centipoises	Section 39-1.02D	Section 39-1.04C			1,500– 4,000	1,500– 4,000
Asphalt modifier	Section 39-1.02D	Section 39-1.04C			Section 39-1.02D	Section 39-1.02D
CRM	Section 39-1.02D	Section 39-1.04C			Section 39-1.02D	Section 39-1.02D

^a Determine combined aggregate gradation containing RAP under California Test 367.

- 1. 1/2-inch, 3/8-inch, or no. 4 aggregate grading is used and the specified total paved thickness is at least 0.15 foot.
- 2. 3/4-inch aggregate grading is used and the specified total paved thickness is at least 0.20 foot.
- ^e To determine field compaction use:
 - 1. In-place density measurements using the method specified in your QC plan.
 - 2. California Test 309 to determine the maximum theoretical density at the frequency specified in California Test 375, Part 5C.

^b The tolerances must comply with the allowable tolerances in section 39-1.02E.

^c Report the average of 3 tests from a single split sample.

^d Determine field compaction for any of the following conditions:

^f Determine the bulk specific gravity of each lab-compacted briquette under California Test 308, Method A, and theoretical maximum specific gravity under California Test 309.

⁹ For adjusting the plant controller at the HMA plant.

^hThe Engineer waives this specification if HMA contains 10 percent or less of nonmanufactured sand by weight of total aggregate. Manufactured sand is fine aggregate produced by crushing rock or gravel.

Report only.

Applies to RAP substitution rate greater than 15 percent.

Replace the 1st paragraph of section 39-2.03A with:

02-22-13

The Department samples for acceptance testing and tests for the quality characteristics shown in the following table:

HMA Acceptance—Standard Construction Process

HMA Acceptance—Standard Construction Process								
Qua	ality cha	racteris	stic	Test			A type	
				method	Α	В	RHMA-G	OGFC
Agg	regate (gradatio	on ^a	California	JMF ±	JMF ±	JMF ±	JMF ±
Sieve	3/4"	1/2"	3/8"	Test 202	tolerance ^c	tolerance ^c	tolerance ^c	tolerance ^c
1/2"	Χb							
3/8"		Χ						
No. 4			Χ					
No. 8	Χ	Χ	Χ					
No. 200	Х	Х	Х					
Sand equ	uivalent	(min) ^c		California Test 217	47	42	47	
Asphalt b	oinder c	ontent	(%)	California Test 379 or 382	JMF±0.40	JMF±0.40	JMF ± 0.40	JMF ± 0.40
HMA mo (%, max)		ontent		California Test 226 or 370	1.0	1.0	1.0	1.0
Field con theoretic			ax.	California Test 375	91–97	91–97	91–97	
Stabilom			1) ^{d,}	California				
No. 4 and 3/8" gradings			Test 366	30	30			
1/2" and 3/4" gradings				37	35	23		
Air void content (%) d, g		California Test 367	4 ± 2	4 ± 2	TV ± 2			
Coarse a One Two Fine agg (Pas	Percent of crushed particles Coarse aggregate (%, min) One fractured face Two fractured faces Fine aggregate (%, min) (Passing no. 4 sieve and		California Test 205	90 75	25 	 90	90 75	
One	ned on fracture	ed face	,		70	20	70	90
Loss	at 100 at 500	rev. rev.	,	California Test 211	12 45	 50	12 40	12 40
Fine agg min) ^h	regate a	angulaı	rity (%,	California Test 234	45	45	45	
Flat and (%, max				California Test 235	Report only	Report only	Report only	Report only
Voids filled with asphalt (%) No. 4 grading 3/8" grading 1/2" grading 3/4" grading		California Test 367	65.0–75.0 65.0–75.0 65.0–75.0 65.0–75.0	65.0–75.0 65.0–75.0 65.0–75.0 65.0–75.0	Report only			
(% min) i No. 4 3/8" (1/2" (3/4" (Voids in mineral aggregate (% min) ⁱ No. 4 grading 3/8" grading 1/2" grading 3/4" grading		California Test 367 California	17.0 15.0 14.0 13.0	17.0 15.0 14.0 13.0	 18.0–23.0 18.0–23.0 Report only		
Dust proportion '			Danionia			i report only		

No. 4 and 3/8" gradings	Test 367	0.6-1.2	0.6-1.2		
1/2" and 3/4" gradings		0.6–1.2	0.6–1.2		
Hamburg wheel track	AASHTO				
(minimum number of passes at	T 324				
0.5 inch average rut depth) ^j	(Modified)				
PG-58		10,000	10,000		
PG-64		15,000	15,000		
PG-70		20,000	20,000		
PG-76 or higher		25,000	25,000		
Hamburg wheel track	AASHTO				
(inflection point minimum	T 324				
number of passes)	(Modified)				
PG-58	,	10,000	10,000		
PG-64		10,000	10,000		
PG-70		12,500	12,500		
PG-76 or higher		15000	15000		
Moisture susceptibility	California	100	100		
(minimum dry strength, psi) ^j	Test 371	120	120		
Moisture susceptibility	California	70	70		
(tensile strength ration, %) ^j	Test 371	70	70		
Smoothness	Section	12-foot	12-foot	12-foot	12-foot
	39-1.12	straight-	straight-	straight-	straight-
		edge,	edge, must	edge, must	edge and
		must	grind, and	grind, and	must grind
		grind, and	Pl_0	Pl₀	, and the second
		Pl ₀			
Asphalt binder	Various	Section 92	Section 92	Section 92	Section 92
Asphalt rubber binder	Various			Section	Section
'				92-	92-1.01D(2)
				1.01D(2)	and section
				and section	39-1.02D
				39-1.02D	
Asphalt modifier	Various			Section	Section
				39-1.02D	39-1.02D
CRM	Various			Section	Section
				39-1.02D	39-1.02D

^a The Engineer determines combined aggregate gradations containing RAP under California Test 367.

- 1. California Test 308, Method A, to determine in-place density of each density core.
- 2. California Test 309 to determine the maximum theoretical density at the frequency specified in California Test 375, Part 5C.

^b "X" denotes the sieves the Engineer tests for the specified aggregate gradation.

^c The tolerances must comply with the allowable tolerances in section 39-1.02E.

^d The Engineer reports the average of 3 tests from a single split sample.

^e The Engineer determines field compaction for any of the following conditions:

^{1. 1/2-}inch, 3/8-inch, or no. 4 aggregate grading is used and the specified total paved thickness is at least 0.15 foot.2. 3/4-inch aggregate grading is used and the specified total paved thickness is at least 0.20 foot.

^f To determine field compaction, the Engineer uses:

⁹The Engineer determines the bulk specific gravity of each lab-compacted briquette under California Test 308, Method A, and theoretical maximum specific gravity under California Test 309.

^h The Engineer waives this specification if HMA contains 10 percent or less of nonmanufactured sand by weight of total aggregate. Manufactured sand is fine aggregate produced by crushing rock or gravel.

Report only.

Applies to RAP substitution rate greater than 15 percent.

Replace the 5th paragraph of section 39-2.03A with:

01-20-12

The Engineer determines the percent of maximum theoretical density from density cores taken from the final layer measured the full depth of the total paved HMA thickness if any of the following applies:

- 1. 1/2-inch, 3/8-inch, or no. 4 aggregate grading is used and the specified total paved thickness is at least 0.15 foot and any layer is less than 0.15 foot.
- 2. 3/4-inch aggregate grading is used and the specified total paved thickness is at least 0.2 foot and any layer is less than 0.20 foot.

Replace the 1st paragraph of section 39-3.02A with:

02-22-13

The Department samples for acceptance testing and tests for the quality characteristics shown in the following table:

HMA Acceptance—Method Construction Process

	ethod Const					
				OGFC		
	JMF ± [JMF ± [JMF ± [JMF ± [
	tolerance ^o	tolerance ^D	tolerance ^D	tolerance ^b		
	47	42	47			
	JMF±0.40	JMF±0.40	JMF ± 0.40	JMF ± 0.40		
	1.0	1.0	1.0	1.0		
Test 366						
0 111	37	35	23			
Test 205	00	0.5		00		
				90		
	75		90	75		
	70	20	70	90		
California	70	20	70	30		
	12		12	12		
1631211		50		40		
California						
Test 367	4 ± 2	4 ± 2	IV ± 2			
California	4E	4E	4E			
Test 234	45	45	45			
California	Report	Roport only	Roport only	Report only		
Test 235	only	neport only	neport only	neport only		
Test 367						
			Report only			
			Troport orny			
0 "' '	65.0-/5.0	65.0-/5.0				
1 est 367	17.0	17.0				
			10 0 00 0			
California	13.0	13.0	10.0-23.0			
	06.12	06.12	Roport only	_		
1631307			i report offiny			
ΔΔΩΗΤΩ	0.0-1.2	0.0-1.2				
(Wickinga)	40.000	40.000				
	10,000	10,000				
	Test method California Test 202 California Test 217 California Test 379 or 382 California Test 226 or 370 California Test 366 California Test 205 California Test 205 California Test 205	Test method A California Test 202 California Test 217 California Test 379 or 382 California Test 226 or 370 California Test 205 California Test 205 California Test 205 California Test 211 California Test 211 California Test 367	Test method A B California Test 202 tolerance toleranc	method A B RHMA-G California Test 202 JMF ± tolerance b JMF ± tolerance b tolerance b tolerance b tolerance b tolerance b California Test 217 JMF±0.40 JMF±0.40 JMF±0.40 Test 379 or 382 JMF±0.40 JMF±0.40 JMF±0.40 California Test 226 or 370 1.0 1.0 1.0 California Test 366 30 30 California Test 205 90 25 70 20 70 California Test 367 4±2 4±2 TV±2 California Test 234 45 45 45 California Test 235 Report only Report only Report only California Test 367 Report only Report only Report only California Test 367 17.0 California Test 367 17.0 17.0 California Test 367 15.0 California Test 367 15.0		

PG-70		20,000	20,000		
PG-76 or higher		25,000	25,000		
Hamburg wheel track	AASHTO				
(inflection point minimum	T 324				
number of passes) ^g	(Modified)				
PG-58		10,000	10,000		
PG-64		10,000	10,000		
PG-70		12,500	12,500		
PG-76 or higher		15000	15000		
Moisture susceptibility	California	120	120		
(minimum dry strength, psi) ^g	Test 371	120	120		
Moisture susceptibility	California	70	70		
(tensile strength ration, %) ⁹	Test 371	70	70		
Smoothness	Section	12-foot	12-foot	12-foot	12-foot
	39-1.12	straight-	straight-	straight-	straight-
		edge and	edge and	edge and	edge and
		must-grind	must-grind	must-grind	must-grind
Asphalt binder	Various	Section 92	Section 92	Section 92	Section 92
Asphalt rubber binder	Various			Section	Section
				92-	92-
				1.01D(2)	1.01D(2)
				and section	and section
				39-1.02D	39-1.02D
Asphalt modifier	Various			Section	Section
				39-1.02D	39-1.02D
CRM	Various			Section	Section
				39-1.02D	39-1.02D

^a The Engineer determines combined aggregate gradations containing RAP under California Test 367.

Replace "280 degrees F" in item 2 in the list in the 6th paragraph of section 39-3.04 with:

01-20-12

285 degrees F

Replace "5,000" in the 5th paragraph of section 39-4.02C with:

02-22-13

10,000

Replace the 7th paragraph of section 39-4.02C with:

02-22-13

Except for RAP substitution rate of greater than 15 percent, the Department does not use results from California Test 371 to determine specification compliance.

^b The tolerances must comply with the allowable tolerances in section 39-1.02E.

^c The Engineer reports the average of 3 tests from a single split sample.

^d The Engineer determines the bulk specific gravity of each lab-compacted briquette under California Test 308, Method A, and theoretical maximum specific gravity under California Test 309.

^e The Engineer waives this specification if HMA contains 10 percent or less of nonmanufactured sand by weight of total aggregate. Manufactured sand is fine aggregate produced by crushing rock or gravel.

Report only.

⁹ Applies to RAP substitution rate greater than 15 percent.

Replace the 8th paragraph of section 39-4.02C with:

02-22-13

Comply with the values for the HMA quality characteristics and minimum random sampling and testing for quality control shown in the following table:

Minimum Quality Control—QC/QA Construction Process							
Quality characteristic	Test method	Minimum sampling and		НМА Туре		Location of sampling	Maxi- mum report
		testing frequency	A	В	RHMA-G		-ing time allow- ance
Aggregate gradation ^a Asphalt binder content (%)	California Test 202 California Test 379 or 382	1 per 750 tons	JMF ± tolerance b JMF±0.40	JMF ± tolerance b JMF±0.40	JMF ± tolerance b JMF ±0.40	California Test 125 Loose mix behind paver See California Test 125	24 hours
Field compaction (% max. theoretical density) ^{c,d}	QC plan		92–96	92–96	91–96	QC plan	
Aggregate moisture content at continuous mixing plants and RAP moisture content at continuous mixing plants and batch mixing plants ^e	California Test 226 or 370	2 per day during production	1	-1		Stock- piles or cold feed belts	1
Sand equivalent (min) ^f	California Test 217	1 per 750 tons	47	42	47	California Test 125	24 hours
HMA moisture content (%,max)	California Test 226 or 370	1 per 2,500 tons but not less than 1 per paving day	1.0	1.0	1.0	Loose Mix Behind	24 hours
Stabilometer value (min) ^f No. 4 and 3/8" gradings 1/2" and 3/4" gradings	California Test 366	1 per 4,000 tons or 2 per 5 business days, whichever	30 37	30 35	 23	Paver See California Test 125	48 hours
Air void content (%) ^{f,g}	California Test 367	is greater	4 ± 2	4 ± 2	TV ± 2		

Davisant of			I				
Percent of crushed							
particles							
coarse							
aggregate							
(% min.):							
One							
fractured			00	05			
face			90	25			
Two							
fractured	California		75		00	California	
faces	Test 205		75		90	Test 125	
Fine							
aggregate							
(% min)							
(Passing no.							
4 sieve and							
retained on							
no. 8 sieve):							
One							
fractured			70	20	70		
face							
Los Angeles Rattler (%							
max):							
Loss at 100	California	As desig-	12		12	California	
rev.	Test 211	nated in	12		12	Test 125	
Loss at 500		QC plan.	45	50	40		
rev.		QO pian.	10	00	40		48
Fine		At least					hours
aggregate	California	once per	45	45	45	California	
angularity	Test 234	project.	45	45	45	Test 125	
(% min) h							
Flat and							
elongated							
particle	California		Report	Report	Report	California	
(% max by	Test 235		only	only	only	Test 125	
weight @							
5:1)	0 111						
Voids filled	California						
with asphalt	Test 367						
(%)							
No. 4 grading					Report		
3/8" grading			65.0-75.0	65.0–75.0	only		
1/2" grading			65.0-75.0	65.0-75.0			
3/4" grading			65.0-75.0	65.0-75.0			
			65.0-75.0	65.0 - 75.0			
Voids in	California						
mineral	Test 367						
aggregate							
(% min.) ⁱ							
No. 4 grading							
3/8" grading			17.0	17.0			
1/2" grading			15.0	15.0			
3/4" grading			14.0	14.0	18.0–23.0		
			13.0	13.0	18.0–23.0		

No. 4 and 3/8' gradings 1/2' and 3/4' gradings 1/2' and 3/4' gradings 1 per (Modified) 1 per project whichever is greater 10,000 15,000 25,000 15,000 25,000 16,000 15,0	Б.	0 111 1	T	I		<u> </u>	ı	
No. 4 and 3/8" gradings 1/2" and 3/4"	Dust i	California						
No.4 and order No.4	proportion	Test 367						
No.4 and order No.4	No 4 and					Report		
1/2" and 3/4" gradings				0610	0610			
Hamburg Modified Task (minimum number of passes) PG-58 PG-64 PG-70 PG-76 or higher Modified PG-70 PG-76 or higher ASHTO Task (inflection point minimum number of passes) PG-58 PG-64 PG-70 PG-76 or higher Modified Test 371 (minimum numbur of passes) PG-70 PG-76 or higher Task (inflection point minimum number of passes) PG-70 PG-76 or higher Task (inflection point minimum number of passes) PG-70 PG-76 or higher Task (inflection point minimum number of passes) PG-70 PG-76 or higher Test 371 (minimum dry strength, psi) Test 371 Ter 10,000 tons or 1 per project whichever is greater Test 371 (tensile strength ratio, %) Test 371 Ter 10,000 tons or 1 per project whichever is greater Test 371 (tensile strength ratio, %) Test 371 Ter 10,000 tons or 1 per project whichever is greater Test 371 (tensile strength ratio, %) Test 371 Ter 10,000 tons or 1 per project whichever is greater Test 371 (tensile strength ratio, %) Test 371 Ter 10,000 tons or 1 per project whichever is greater Test 371 (tensile strength ratio, %) Test 371 Ter 10,000 tons or 1 per project whichever is greater Test 371 (tensile strength ratio, %) Test 371 Ter 10,000 tons or 1 per project whichever is greater Test 371 (tensile strength ratio, %) Tes				0.0-1.2	0.0-1.2	-		
Hamburg wheel track (minimum number of passes at 0.5 inch average rut depth) PG-58 PG-64 PG-70 PG-70 PG-70 PG-70 PG-88 PG-84 PG-70 PG-88 PG-84 PG-70 PG-88 PG-84 PG-70 PG-88 PG-84 PG-70 PG-70 PG-70 PG-70 PG-70 PG-88 PG-84 PG-70 PG-				0612	0610			
wheel track (minimum number of passes at 0.5 1 per (Modified) 10,000 10,000 10,000 15,000 25,000		AACUTO		0.0-1.2	0.0-1.2			
(minimum number of passes at 0.5 inch average rut depth) ⁱⁱ PG-58 PG-64 PG-70 PG-70 PG-70 PG-76 or higher (Modified) 10,000 15,000 20,00			1 225					
Number of passes at 0.5 per project whichever is greater 10,000 10,000 20,								
passes at 0.5 inch average rut depth) ¹ PG-58 PG-64 PG-70 PG-76 or higher Hamburg wheel track (inflection point minimum number of passes) ¹ PG-58 PG-64 PG-70 PG-76 or higher higher whichever is greater loop to sor 1 per project whichever is greater loop tons or 1 per project whichever loop tons or 1 per projec		(iviodilled)						
inch average rut depth)¹ PG-58 PG-64 PG-70 PG-76 or higher Inmimum rum dry strength, psi)¹ PG-76 or higher Susceptibility (tensile strength ratio, %)¹ PG-12 PG-76 or higher Smoothness Smoothness Smoothness Smoothness Smoothness Section 39-1.12 Section 39-1.02D Section 48 Asphalt rubber bioled viscosity ⊕ 375 = F, centipoises CRM Section 39-1.02D Section 15,000 110,000 110,000 15,000 15,000 2								
rut depth) ⁱⁱ PG-58 PG-64 PG-70 PG-76 or higher Hamburg wheel track (inflection point minimum number of passes) ⁱ PG-76 or higher PG-76 or higher Moisture susceptibility (minimum dry strength, psi) ⁱ PG-87 Test 371 T								
PG-58 PG-64 PG-70 PG-70 PG-76 or higher AASHTO T 324 (Modified) 1 per 10,000 20,000 25,000 26,000								
PG-64 PG-70 PG-76 or higher AASHTO wheel track (inflection point minimum number of passes) PG-58 PG-64 PG-70 PG-76 or higher PG-70 PG-70 PG-76 or higher PG-70			is greater	10.000	10 000			
PG-76 or higher								
PG-76 or higher Hamburg Hamburg Modified T 324 (Modified) T 324 T per project whichever is greater T 10,000								
Hamburg				20,000	20,000			
Hamburg wheel track (inflection point minimum number of passes)				25.000	25.000			
Wheel track (inflection point point minimum number of passes)		AASHTO						
(inflection point minimum minim			1 per					
Doint minimum Doint minimu								
Description		(**************************************						
number of passess)¹ Whichever is greater 10,000 10,000 10,000 10,000 10,000 10,000 10,000 10,000 10,000 12,500 1								
PG-58								
PG-58	passes) ^j		is greater					
PG-70				10,000	10,000			
PG-76 or higher	PG-64			10,000	10,000			
Moisture Susceptibility (minimum dry strength, psi) Moisture California Test 371 1 per 10,000 tons or 1 120 120	PG-70			12,500	12,500			
Moisture susceptibility (minimum dry strength, psi) Test 371	PG-76 or							
Susceptibility (minimum dry strength, psi) Susceptibility (minimum dry strength, psi) Susceptibility (tensile strength ratio, %) Section 39-1.12 Section 39-1.02D Section 39-1.02D Section CRM Section				15000	15000			
(minimum dry strength, psi) j10,000 tons or 1 per project whichever is greater120120Moisture susceptibility (tensile strength ratio, %) jCalifornia Test 3711 per 10,000 tons or 1 per project whichever is greater707070SmoothnessSection 39-1.1212-foot straight- edge, must- grind, and Plo12-foot straight- edge, must- grind, and Plo12-foot straight- edge, must- grind, and Plo12-foot straight- edge, must- grind, and PloAsphalt rubber binder viscosity @ 375 °F, centipoisesSection 39-1.02D1,500- 4,000Section 39-1.02D24 hoursCRMSectionSection 39-1.02D39-1.02D								
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Moisture susceptibility (tensile strength ratio, %)				120	120			
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375 °F, centipoises								
centipoisesSectionSection48		39-1.02D				4,000	39-1.02D	hours
CRM Section Section Section 48								
		Section				Section	Section	48
TO THE THOUSE IN THE PROPERTY OF THE PROPERTY		39-1.02D				39-1.02D	39-1.02D	hours

^b The tolerances must comply with the allowable tolerances in section 39-1.02E.

- 1. In-place density measurements using the method specified in your QC plan.
- 2. California Test 309 to determine the maximum theoretical density at the frequency specified in California Test 375, Part 5C.

Replace the 1st sentence in the 1st paragraph of section 39-4.03B(2) with:

01-20-12

For aggregate gradation and asphalt binder content, the minimum ratio of verification testing frequency to quality control testing frequency is 1:5.

Replace the 2nd "and" in the 7th paragraph of section 39-4.03B(2) with:

01-20-12

or

^a Determine combined aggregate gradation containing RAP under California Test 367.

^c Determines field compaction for any of the following conditions:

^{1. 1/2-}inch, 3/8-inch, or no. 4 aggregate grading is used and the specified total paved thickness is at least 0.15 foot.2. 3/4-inch aggregate grading is used and the specified total paved thickness is at least 0.20 foot.

^d To determine field compaction use:

^e For adjusting the plant controller at the HMA plant.

Report the average of 3 tests from a single split sample.

⁹ Determine the bulk specific gravity of each lab-compacted briquette under California Test 308, Method A, and theoretical maximum specific gravity under California Test 309.

^h The Engineer waives this specification if HMA contains 10 percent or less of nonmanufactured sand by weight of total aggregate. Manufactured sand is fine aggregate produced by crushing rock or gravel.

Report only.

Applies to RAP substitution rate greater than 15 percent.

Replace the 1st paragraph of section 39-4.04A with:

02-22-13

The Engineer samples for acceptance testing and tests for the following quality characteristics:

HMA Acceptance—QC/QA Construction Process

	HMA Acceptance—QC/QA Construction Process									
Index	Qua	ality cha	aracteri	stic	Weight	Test		HMA type	MA type	
(i)					-ing	method				
					factor		Α	В	RHMA-G	
					(w)					
		Α	Aggrega	ate						
		g	radatio	n ^a						
	Sieve	3/4"	1/2"	3/8"						
1	1/2"	Χb			0.05	California		MF ± Tolerand	C C	
1	3/8"		Х		0.05	Test 202	3	ivii ± roleiand	.6	
1	No. 4			Χ	0.05					
2	No. 8	Χ	Χ	Χ	0.10					
3	No.	Х	Х	Х	0.15					
	200									
4	Asphal	t binder	conter	nt (%)	0.30	California	JMF±0.40	JMF±0.40	JMF ± 0.40	
						Test 379				
						or 382				
5	Field co			max.	0.40	California	92–96	92–96	91–96	
	theoret			e		Test 375				
	Sand e	quivale	nt (min) †		California	47	42	47	
	. , ,				Test 217					
	Stabilometer value (min) [†]				California					
	No. 4 and 3/8" gradings				Test 366	30	30			
	1/2" and 3/4" gradings					37	35	23		
	Air void content (%) ^{t, g}				California	4 ± 2	4 ± 2	TV ± 2		
					Test 367					
	Percent of crushed particles				California					
	coarse					Test 205				
		e fractu					90	25		
		o fractu					75		90	
	Fine ag									
		assing r								
		d retain	ed on N	No. 8						
		ve.)							70	
		e fractu					70	20	70	
	HMA m		conter	nt		California	1.0	1.0	1.0	
	(%, ma	X)				Test 226				
	1.55 4	ada: F) a #1 a == /-	0/		or 370				
1	Los Angeles Rattler (%				California					
	max)				Test 211	10		10		
	Loss at 100 rev.					12 45	50	12 40		
	Loss at 500 rev.				California	45 45	45	45		
	Fine aggregate angularity				Test 234	45	45	40		
 	(% min) ⁿ				California	Report	Report only	Report only		
	Flat and elongated particle				Test 235	only	i report only	ineport only		
	(% max by weight @ 5:1) Voids in mineral aggregate				California	Offig				
1	(% min		aı ayyı	eyale		Test 367				
		<i>)</i> . 4 grac	lina			1691 307	17.0	17.0		
1		. 4 gradir " gradir	_				15.0	15.0	18.0–23.0	
1		" gradir					14.0	14.0	18.0–23.0	
		" gradir					13.0	13.0	10.0 20.0	
	5	grauli	'9				10.0	10.0		

Voids filled with asphalt (%)	California Test 367			
No. 4 grading 3/8" grading 1/2" grading 3/4" grading	1631667	65.0–75.0 65.0–75.0 65.0–75.0 65.0–75.0	65.0–75.0 65.0–75.0 65.0–75.0 65.0–75.0	Report only
Dust proportion No. 4 and 3/8" gradings 1/2" and 3/4" gradings	California Test 367	0.6–1.2 0.6–1.2	0.6–1.2 0.6–1.2	Report only
Hamburg Wheel Tracker (minimum number of passes at 0.5 inch average rut depth) ^j PG-58 PG-64 PG-70 PG-76 or higher	AASHTO T 324 (Modified)	10,000 15,000 20,000 25,000	10,000 15,000 20,000 25,000	
Hamburg Wheel Tracker (inflection point minimum number of passes) ^j PG-58 PG-64 PG-70 PG-76 or higher	AASHTO T 324 (Modified)	10,000 15,000 20,000 25,000	10,000 15,000 20,000 25,000	
Moisture susceptibility (minimum dry strength, psi) j	California Test 371	120	120	
Moisture susceptibility (tensile strength ratio %) ^j	California Test 371	70	70	70
Smoothness	Section 39-1.12	12-foot straight- edge, must grind, and PI ₀	12-foot straight- edge, must grind, and PI ₀	12-foot straight- edge, must grind, and Pl ₀
Asphalt binder	Various	Section 92	Section 92	Section 92
Asphalt rubber binder	Various			Section 92-1.01D(2) and section 39-1.02D
Asphalt modifier	Various			Section 39-1.02D
CRM	Various			Section 39-1.02D

b "X" denotes the sieves the Engineer tests for the specified aggregate gradation.

^e To determine field compaction, the Engineer uses:

- 1. California Test 308, Method A, to determine in-place density of each density core.
- 2. California Test 309 to determine the maximum theoretical density at the frequency specified in California Test 375, Part 5C.

^f The Engineer reports the average of 3 tests from a single split sample.

Report only.

Replace the 3rd paragraph of section 39-4.04A with:

01-20-12

The Department determines the percent of maximum theoretical density from density cores taken from the final layer measured the full depth of the total paved HMA thickness if any of the following applies:

- 1. 1/2-inch, 3/8-inch, or no. 4 aggregate grading is used and the specified total paved thickness is at least 0.15 foot and any lager is less than 0.15 foot.
- 2. 3/4-inch aggregate grading is used and the specified total paved thickness is at least 0.20 and any layer is less than 0.20 foot.

^^^^^

40 CONCRETE PAVEMENT

01-20-12 **Replace section 40-1.01C(4) with:**

01-20-12

40-1.01C(4) Authorized Laboratory

Submit for authorization the name of the laboratory you propose to use for testing the drilled core specimens for air content.

Replace the paragraph in section 40-1.01C(8) with:

01-20-12

Submit a plan for protecting concrete pavement during the initial 72 hours after paving when the forecasted minimum ambient temperature is below 40 degrees F.

01-20-12

Delete "determined under California Test 559" in section 40-1.01C(9).

^a The Engineer determines combined aggregate gradations containing RAP under California Test 367.

^c The tolerances must comply with the allowable tolerances in section 39-1.02E.

^d The Engineer determines field compaction for any of the following conditions:

^{1. 1/2-}inch, 3/8-inch, or no. 4 aggregate grading is used and the specified total paved thickness is at least 0.15 foot and less than 0.20 foot. 3/4-inch aggregate grading is used and the specified total paved thickness is at least 0.20 foot.

⁹ The Engineer determines the bulk specific gravity of each lab-compacted briquette under California Test 308, Method A, and theoretical maximum specific gravity under California Test 309.

^h The Engineer waives this specification if HMA contains 10 percent or less of nonmanufactured sand by weight of total aggregate. Manufactured sand is fine aggregate produced by crushing rock or gravel.

Applies to RAP substitution rate greater than 15 percent.

Replace the 2nd and 3rd paragraphs in section 40-1.01D(4) with:

01-20-12

The QC plan must include details of corrective action to be taken if any process is out of control. As a minimum, a process is out of control if any of the following occurs:

- 1. For fine and coarse aggregate gradation, 2 consecutive running averages of 4 tests are outside the specification limits
- 2. For individual penetration or air content measurements:
 - 2.1. One point falls outside the suspension limit line
 - 2.2. Two points in a row fall outside the action limit line

Stop production and take corrective action for out of control processes or the Engineer rejects subsequent material.

Replace the 1st paragraph in section 40-1.01D(5) with:

01-20-12

Determine the minimum cementitious materials content. Use your value for minimum cementitious material content for *MC* in equation 1 and equation 2 of section 90-1.02B(3).

Replace the 1st sentence of the 3rd paragraph of section 40-1.01D(9) with:

01-20-12

Use a California profilograph to determine the concrete pavement profile.

Replace the title of the table in section 40-1.01D(13)(a) with:

01-20-12

Concrete Pavement Acceptance Testing

Replace the 2nd and 3rd paragraphs in section 40-1.01D(13)(a) with:

01-20-12

Pavement smoothness may be accepted based on the Department's testing. A single test represents no more than 0.1 mile.

Acceptance of modulus of rupture, thickness, dowel bar and tie bar placement, coefficient of friction, smoothness, and air content, does not constitute final concrete pavement acceptance.

01-20-12

Delete item 4 in the list in the 2nd paragraph in section 40-1.01D(13)(c)(2).

Replace items 1 and 2 in the list in the 2nd paragraph in 40-1.01D(13)(d) with:

01-20-12

- 1. For tangents and horizontal curves having a centerline radius of curvature 2,000 feet or more, the PI₀ must be at most 2-1/2 inches per 0.1-mile section.
- 2. For horizontal curves having a centerline radius of curvature from 1,000 to 2,000 feet including concrete pavement within the superelevation transitions of those curves, the PI₀ must be at most 5 inches per 0.1-mile section.

Replace the 1st and 2nd variables in the equation in section 40-1.01D(13)(f) with:

01-20-12

n_c = Number of your quality control tests (minimum of 6 required)

Replace "Your approved third party independent testing laboratory" in the 4th paragraph of section 40-1.01D(13)(f) with:

01-20-12

The authorized laboratory

Replace item 2 in the list in the 2nd paragraph of section 40-1.01D(13)(g):

01-20-12

2. One test for every 4,000 square yards of concrete pavement with tie bars or remaining fraction of that area. Each tie bar test consists of 2 cores with 1 on each tie-bar-end to expose both ends and allow measurement.

Replace section 40-1.01D(13)(h) with:

01-20-12

40-1.01D(13)(h) Bar Reinforcement

Bar reinforcement is accepted based on inspection before concrete placement.

Replace the paragraph in section 40-1.02B(2) with:

01-20-12

PCC for concrete pavement must comply with section 90-1 except as otherwise specified.

Replace the paragraphs in section 40-1.02D with:

01-20-12

Bar reinforcement must be deformed bars.

If the project is not shown to be in high desert or any mountain climate region, bar reinforcement must comply with section 52.

If the project is shown to be in high desert or any mountain climate regions, bar reinforcement must be one of the following:

- Epoxy-coated bar reinforcement under section 52-2.03B except bars must comply with either ASTM A 706/A 706M; ASTM A 996/A 996M; or ASTM A 615/A 615M, Grade 40 or 60. Bars must be handled under ASTM D 3963/D 3963M and section 52-2.02C.
- 2. Low carbon, chromium steel bar complying with ASTM A 1035/A 1035M

Replace the paragraphs in section 40-1.02E with:

01-20-12

Tie bars must be deformed bars.

If the project is not shown to be in high desert or any mountain climate region, tie bars must be one of the following:

- 1. Epoxy-coated bar reinforcement. Bars must comply with either section 52-2.02B or 52-2.03B except bars must comply with either ASTM A 706/A 706M; ASTM A 996/A 996M; or ASTM A 615/A 615M, Grade 40 or 60.
- 2. Stainless-steel bars. Bars must be descaled, pickled, polished, and solid stainless-steel bars under ASTM A 955/A 955M, Grade 60, UNS Designation S31603 or S31803.
- 3. Low carbon, chromium-steel bars under ASTM A 1035/A 1035M.

If the project is shown to be in high desert or any mountain climate region, tie bars must be one of the following:

- 1. Epoxy-coated bar reinforcement. Bars must comply with section 52-2.03B except bars must comply with either ASTM A 706/A 706M; ASTM A 996/A 996M; or ASTM A 615/A 615M, Grade 40 or 60.
- 2. Stainless-steel bars. Bars must be descaled, pickled, polished, and solid stainless-steel bars under ASTM A 955/A 955M, Grade 60, UNS Designation S31603 or S31803.

Fabricate, sample, and handle epoxy-coated tie bars under ASTM D 3963/D 3963M, section 52-2.02C, or section 52-2.03C.

Do not bend tie bars.

Replace the 1st, 2nd, and 3rd paragraphs in section 40-1.02F with:

01-20-12

Dowel bars must be plain bars. Fabricate, sample, and handle epoxy-coated dowel bars under ASTM D 3963/D 3963M and section 52-2.03C except each sample must be 18 inches long.

If the project is not shown to be in high desert or any mountain climate region, dowel bars must be one of the following:

- 1. Epoxy-coated bars. Bars must comply with ASTM A 615/A 615M, Grade 40 or 60. Epoxy coating must comply with either section 52-2.02B or 52-2.03B.
- 2. Stainless-steel bars. Bars must be descaled, pickled, polished, and solid stainless-steel bars under ASTM A 955/A 955M, Grade 60, UNS Designation S31603 or S31803.
- 3. Low carbon, chromium-steel bars under ASTM A 1035/A 1035M.

If the project is shown to be in high desert or any mountain climate region, dowel bars must be one of the following:

- 1. Epoxy-coated bars. Bars must comply with ASTM A 615/A 615M, Grade 40 or 60. Epoxy coating must comply with section 52-2.03B.
- 2. Stainless-steel bars. Bars must be descaled, pickled, polished, and solid stainless-steel bars under ASTM A 955/A 955M, Grade 60, UNS Designation S31603 or S31803.

Replace the paragraphs in section 40-1.02G with:

01-20-12

For dowel and tie bar baskets, wire must comply with ASTM A 82/A 82M and be welded under ASTM A 185/A 185M, Section 7.4. The minimum wire-size no. is W10. Use either U-frame or A-frame shaped assemblies.

If the project is not shown to be in high desert or any mountain climate region. Baskets may be epoxycoated, and the epoxy coating must comply with either section 52-2.02B or 52-2.03B.

If the project is shown to be in high desert or any mountain climate region, wire for dowel bar and tie bar baskets must be one of the following:

- 1. Epoxy-coated wire complying with section 52-2.03B
- 2. Stainless-steel wire. Wire must be descaled, pickled, and polished solid stainless-steel. Wire must comply with (1) the chemical requirements in ASTM A 276/A 276M, UNS Designation S31603 or S31803 and (2) the tension requirements in ASTM A 1022/ A 1022M.

Handle epoxy-coated tie bar and dowel bar baskets under ASTM D 3963/D 3963M and either section 52-2.02B or 52-2.03B.

Fasteners must be driven fasteners under ASTM F 1667. Fasteners on lean concrete base or HMA must have a minimum shank diameter of 3/16 inch and a minimum shank length of 2-1/2 inches. For asphalt

treated permeable base or cement treated permeable base, the shank diameter must be at least 3/16 inch and the shank length must be at least 5 inches.

Fasteners, clips, and washers must have a minimum 0.2-mil thick zinc coating applied by either electroplating or galvanizing.

Replace the 1st paragraph in section 40-1.02H with:

01-20-12

Chemical adhesive for drilling and bonding dowels and tie bars must be on the Authorized Material List. The Authorized Material List indicates the appropriate chemical adhesive system for the concrete temperature and installation conditions.

Replace section 40-1.02I(2) with:

40-1.02I(2) Silicone Joint Sealant

01-20-12

Silicone joint sealant must be on the Authorized Material List.

Replace the last sentence in section 40-1.02I(4) with:

01-20-12

Show evidence that the seals are compressed from 30 to 50 percent for the joint width at time of installation.

Replace the paragraph in section 40-1.02L with:

01-20-12

Water for core drilling may be obtained from a potable water source, or submit proof that it does not contain:

- 1. More than 1,000 parts per million of chlorides as CI
- 2. More than 1,300 parts per million of sulfates as SO₄
- 3. Impurities that cause pavement discoloration or surface etching

Replace the paragraph in section 40-1.03B with:

01-20-12

Before placing concrete pavement, develop enough water supply for the work under section 17.

Replace the last paragraph in section 40-1.03D(1) with:

01-20-12

Removal of grinding residue must comply with section 42-1.03B.

Replace the 1st and 2nd paragraphs in section 40-1.03E(6)(c) with:

01-20-12

Install preformed compressions seals in isolation joints if specified in the special provisions.

Install longitudinal seals before transverse seals. Longitudinal seals must be continuous except splicing is allowed at intersections with transverse seals. Transverse seals must be continuous for the entire transverse length of concrete pavement except splices are allowed for widenings and staged construction. With a sharp instrument, cut across the longitudinal seal at the intersection with transverse

construction joints. If the longitudinal seal does not relax enough to properly install the transverse seal, trim the longitudinal seal to form a tight seal between the 2 joints.

If splicing is authorized, splicing must comply with the manufacturer's written instructions.

Replace the 12th and 13th paragraphs in section 40-1.03G with:

01-20-12

Construct additional test strips if you:

- 1. Propose different paving equipment including:
 - 1.1. Paver
 - 1.2. Dowel bar inserter
 - 1.3. Tie bar inserter
 - 1.4. Tining
 - 1.5. Curing equipment
- 2. Change concrete mix proportions

You may request authorization to eliminate the test strip if you use paving equipment and personnel from a Department project (1) for the same type of pavement and (2) completed within the past 12 months. Submit supporting documents and previous project information with your request.

Replace the 1st paragraph in section 40-1.03l with:

01-20-12

Place tie bars in compliance with the tolerances shown in the following table:

Tie Bar Tolerance

Dimension	Tolerance
Horizontal and vertical skew	10 degrees maximum
Longitudinal translation	± 2 inch maximum
Horizontal offset (embedment)	± 2 inch maximum
Vertical depth	 Not less than 1/2 inch below the saw cut depth of joints When measured at any point along the bar, not less than 2 inches clear of the pavement's surface and bottom

Replace item 4 in the list in the 2nd paragraph in section 40-1.03l with:

01-20-12

4. Use tie bar baskets. Anchor baskets at least 200 feet in advance of pavement placement activity. If you request a waiver, describe the construction limitations or restricted access preventing the advanced anchoring. After the baskets are anchored and before paving, demonstrate the tie bars do not move from their specified depth and alignment during paving. Use fasteners to anchor tie bar baskets.

Replace "The maximum distance below the depth shown must be 0.05 foot." in the table in section 40-1.03J with:

01-20-12

The maximum distance below the depth shown must be 5/8 inch.

Replace sections 40-1.03L and 40-1.03M with:

01-20-12

40-1.03L Finishing 40-1.03L(1) General

Reserved

40-1.03L(2) Preliminary Finishing

40-1.03L(2)(a) General

Preliminary finishing must produce a smooth and true-to-grade finish. After preliminary finishing, mark each day's paving with a stamp. The stamp must be authorized before paving starts. The stamp must be approximately 1 by 2 feet in size. The stamp must form a uniform mark from 1/8 to 1/4 inch deep. Locate the mark 20 ± 5 feet from the transverse construction joint formed at each day's start of paving and 1 ± 0.25 foot from the pavement's outside edge. The stamp mark must show the month, day, and year of placement and the station of the transverse construction joint. Orient the stamp mark so it can be read from the pavement's outside edge.

Do not apply more water to the pavement surface than can evaporate before float finishing and texturing are completed.

40-1.03L(2)(b) Stationary Side Form Finishing

If stationary side form construction is used, give the pavement a preliminary finish by the machine float method or the hand method.

If using the machine float method:

- 1. Use self-propelled machine floats.
- 2. Determine the number of machine floats required to perform the work at a rate equal to the pavement delivery rate. If the time from paving to machine float finishing exceeds 30 minutes, stop pavement delivery. When machine floats are in proper position, you may resume pavement delivery and paving.
- Run machine floats on side forms or adjacent pavement lanes. If running on adjacent pavement, protect the adjacent pavement surface under section 40-1.03P. Floats must be hardwood, steel, or steel-shod wood. Floats must be equipped with devices that adjust the underside to a true flat surface.

If using the hand method, finish pavement smooth and true to grade with manually operated floats or powered finishing machines.

40-1.03L(2)(c) Slip-Form Finishing

If slip-form construction is used, the slip-form paver must give the pavement a preliminary finish. You may supplement the slip-form paver with machine floats.

Before the pavement hardens, correct pavement edge slump in excess of 0.02 foot exclusive of edge rounding.

40-1.03L(3) Final Finishing

After completing preliminary finishing, round the edges of the initial paving widths to a 0.04-foot radius. Round transverse and longitudinal construction joints to a 0.02-foot radius.

Before curing, texture the pavement. Perform initial texturing with a burlap drag or broom device that produces striations parallel to the centerline. Perform final texturing with a steel-tined device that produces grooves parallel with the centerline.

Construct longitudinal grooves with a self-propelled machine designed specifically for grooving and texturing pavement. The machine must have tracks to maintain constant speed, provide traction, and maintain accurate tracking along the pavement surface. The machine must have a single row of rectangular spring steel tines. The tines must be from 3/32 to 1/8 inch wide, on 3/4-inch centers, and must have enough length, thickness, and resilience to form grooves approximately 3/16 inch deep. The machine must have horizontal and vertical controls. The machine must apply constant down pressure on the pavement surface during texturing. The machines must not cause ravels.

Construct grooves over the entire pavement width in a single pass except do not construct grooves 3 inches from the pavement edges and longitudinal joints. Final texture must be uniform and smooth. Use a guide to properly align the grooves. Grooves must be parallel and aligned to the pavement edge across the pavement width. Grooves must be from 1/8 to 3/16 inch deep after the pavement has hardened.

For irregular areas and areas inaccessible to the grooving machine, you may hand-construct grooves under section 40-1.03L(2) using the hand method. Hand-constructed grooves must comply with the specifications for machine-constructed grooves.

Initial and final texturing must produce a coefficient of friction of at least 0.30 when tested under California Test 342. Notify the Engineer when the pavement is scheduled to be opened to traffic to allow at least 25 days for the Department to schedule testing for coefficient of friction. Notify the Engineer when the pavement is ready for testing which is the latter of:

- 1. Seven days after paving
- 2. When the pavement has attained a modulus of rupture of 550 psi

The Department tests for coefficient of friction within 7 days of receiving notification that the pavement is ready for testing.

Do not open the pavement to traffic unless the coefficient of friction is at least 0.30.

40-1.03M Reserved

Replace the 4th paragraph of 40-1.03P with:

01-20-12

Construct crossings for traffic convenience. If authorized, you may use RSC for crossings. Do not open crossings until the Department determines that the pavement's modulus of rupture is at least 550 psi under California Test 523 or California Test 524.

Replace the 1st paragraph of section 40-6.01A with:

01-20-12

Section 40-6 includes specifications for applying a high molecular weight methacrylate resin system to pavement surface cracks that do not extend the full slab depth.

Replace the 4th paragraph of section 40-6.01C(2) with:

01-20-12

If the project is in an urban area adjacent to a school or residence, the public safety plan must also include an airborne emissions monitoring plan prepared by a CIH certified in comprehensive practice by the American Board of Industrial Hygiene. Submit a copy of the CIH's certification. The CIH must monitor the emissions at a minimum of 4 points including the mixing point, the application point, and the point of nearest public contact. At work completion, submit a report by the industrial hygienist with results of the airborne emissions monitoring plan.

01-20-12

Delete the 1st sentence of the 2nd paragraph in section 40-6.02B.

Replace item 4 in the list in the last paragraph in section 40-6.03A with:

01-20-12

4. Coefficient of friction is at least 0.30 under California Test 342

Replace the paragraph in section 40-6.04 with:

Not Used

Add to section 40:

01-20-12

40-7-40-15 RESERVED

^^^^^^

41 CONCRETE PAVEMENT REPAIR

10-19-12

Replace "41-1.02" in the 1st paragraph of section 41-3.02 with:

10-19-12

Add to section 41-4.03:

10-19-12

41-4.03J-41-4.03M Reserved

Replace "41-8" in the 3rd paragraph of section 41-7.03 with:

10-19-12

41-9 except

^^^^^

DIVISION VI STRUCTURES 46 GROUND ANCHORS AND SOIL NAILS

01-18-13

Replace the 1st paragraph of section 46-1.01C(2) with:

01-18-13

Submit 5 sets of shop drawings to OSD, Documents Unit. Notify the Engineer of the submittal. Include in the notification the date and contents of the submittal. Allow 30 days for the Department's review. After review, submit from 6 to 12 sets, as requested, for authorization and use during construction.

Shop drawings and calculations must be sealed and signed by an engineer who is registered as a civil engineer in the State.

Replace the 3rd paragraph of section 46-1.01C(2) with:

01-18-13

Ground anchor shop drawings must include:

- 1. Details and specifications for the anchorage system and ground anchors.
- 2. Details for the transition between the corrugated plastic sheathing and the anchorage assembly.
- 3. If shims are used during lock-off, shim thickness and supporting calculations.
- 4. Calculations for determining the bonded length. Do not rely on any capacity from the grout-to-ground bond within the unbonded length.

01-18-13

Delete the 5th and 6th paragraphs of section 46-1.01C(2).

Replace the 4th paragraph of section 46-1.01D(2)(b) with:

01-18-13

Each jack and its gage must be calibrated as a unit under the specifications for jacks used to tension prestressing steel permanently anchored at 25 percent or more of its specified minimum ultimate tensile strength in section 50-1.01D(3).

10-19-12

Delete the 3rd paragraph of section 46-1.01D(2)(d).

Add to section 46-1.03B:

04-20-12

Dispose of drill cuttings under section 19-2.03B.

Replace the 1st sentence of the 3rd paragraph of section 46-2.01A with:

04-20-12

Ground anchors must comply with section 50.

Add to section 46-2.02B:

04-20-12

Strand tendons, bar tendons, bar couplers, and anchorage assemblies must comply with section 50.

^^^^^

47 EARTH RETAINING SYSTEMS

10-19-12

Replace the 2nd paragraph of section 47-2.01D with:

02-17-12

Coupler test samples must comply with minimum tensile specifications for steel wire in ASTM A 82/A 82M. Total wire slip must be at most 3/16 inch when tested under the specifications for tension testing of round wire test samples in ASTM A 370.

Replace "78-80" in the 1st table in the 2nd paragraph of section 47-2.02C with:

10-19-12

78-100

Replace the value for the sand equivalent requirement in the 2nd table in the 3rd paragraph of section 47-2.02C with:

01-20-12

12 minimum

Replace the 1st paragraph of section 47-2.02E with:

02-17-12

Steel wire must comply with ASTM A 82/A 82M. Welded wire reinforcement must comply with ASTM A 185/A 185M.

Add between the 2nd and 3rd paragraphs of section 47-3.02A:

Reinforcement must comply with section 52.

10-19-12

Delete the 1st paragraph of section 47-3.02B(2)(b).

10-19-12

Add between the 3rd and 4th paragraphs of section 47-5.01:

10-19-12

Reinforcement must comply with section 52.

Add to section 47-6.01A:

10-19-12

The alternative earth retaining system must comply with the specifications for the type of wall being constructed.

48 TEMPORARY STRUCTURES

09-16-11

Replace the 7th paragraph of section 48-2.01C(2) with:

09-16-11

If you submit multiple submittals at the same time or additional submittals before review of a previous submittal is complete:

- 1. You must designate a review sequence for submittals
- 2. Review time for any submittal is the review time specified plus 15 days for each submittal of higher priority still under review

^^^^^

49 PILING

01-18-13

Replace "Load Applied to Pile by Hydraulic Jack(s) Acting at One End of Test Beam(s) Anchored to the Pile" in the 5th paragraph of section 49-1.01D(2) with:

07-20-12

"Tensile Load Applied by Hydraulic Jack(s) Acting Upward at One End of Test Beam(s)"

Add to section 49-1.03:

Dispose of drill cuttings under section 19-2.03B.

04-20-12

Replace the 2nd paragraph of section 49-2.01D with:

01-20-12

Furnish piling is measured along the longest side of the pile from the specified tip elevation shown to the plane of pile cutoff.

Replace the 3rd and 4th paragraphs of section 49-2.04B(2) with:

10-19-12

Piles in a corrosive environment must be steam or water cured under section 90-4.03.

If piles in a corrosive environment are steam cured, either:

- 1. Keep the piles continuously wet for at least 3 days. The 3 days includes the holding and steam curing periods.
- 2. Apply curing compound under section 90-1.03B(3) after steam curing.

Add to section 49-3.01A:

01-20-12

Concrete must comply with section 51.

Replace the 1st paragraph of section 49-3.01C with:

01-20-12

Except for CIDH concrete piles constructed under slurry, construct CIP concrete piles such that the excavation methods and the concrete placement procedures provide for placing the concrete against undisturbed material in a dry or dewatered hole.

Replace "Reserved" in section 49-3.02A(2) with:

01-20-12

dry hole:

- 1. Except for CIDH concrete piles specified as end bearing, a drilled hole that:
 - Accumulates no more than 12 inches of water in the bottom of the drilled hole during a period of 1 hour without any pumping from the hole during the hour.
 - Has no more than 3 inches of water in the bottom of the drilled hole immediately before placing concrete.
- 2. For CIDH concrete piles specified as end bearing, a drilled hole free of water without the use of pumps.

Replace "Reserved" in section 49-3.02A(3)(a) with:

01-20-12

If plastic spacers are proposed for use, submit the manufacturer's data and a sample of the plastic spacer. Allow 10 days for review.

Replace item 5 in the list in the 1st paragraph of section 49-3.02A(3)(b) with:

10-19-12

- 5. Methods and equipment for determining:
 - 5.1. Depth of concrete
 - 5.2. Theoretical volume of concrete to be placed, including the effects on volume if casings are withdrawn
 - 5.3. Actual volume of concrete placed

Add to the list in the 1st paragraph of section 49-3.02A(3)(b):

01-18-13

8. Drilling sequence and concrete placement plan.

Replace item 2 in the list in the 1st paragraph of section 49-3.02A(3)(g) with:

01-20-12

- 2. Be sealed and signed by an engineer who is registered as a civil engineer in the State. This requirement is waived for either of the following conditions:
 - 2.1. The proposed mitigation will be performed under the current Department-published version of *ADSC Standard Mitigation Plan 'A' Basic Repair* without exception or modification.
 - 2.2. The Engineer determines that the rejected pile does not require mitigation due to structural, geotechnical, or corrosion concerns, and you elect to repair the pile using the current Department-published version of *ADSC Standard Mitigation Plan 'B' Grouting Repair* without exception or modification.

Replace item 1 in the list in the 1st paragraph of section 49-3.02A(4)(d)(ii) with:

01-20-12

 Inspection pipes must be schedule 40 PVC pipe complying with ASTM D 1785 with a nominal pipe size of 2 inches. Watertight PVC couplers complying with ASTM D 2466 are allowed to facilitate pipe lengths in excess of those commercially available. Log the location of the inspection pipe couplers with respect to the plane of pile cutoff.

Add to section 49-3.02A(4)(d)(iv):

01-20-12

If the Engineer determines it is not feasible to use one of ADSC's standard mitigation plans to mitigate the pile, schedule a meeting and meet with the Engineer before submitting a nonstandard mitigation plan.

The meeting attendees must include your representatives and the Engineer's representatives involved in the pile mitigation. The purpose of the meeting is to discuss the type of pile mitigation acceptable to the Department.

Provide the meeting facility. The Engineer conducts the meeting.

Replace the 1st paragraph of section 49-3.02B(5) with:

01-20-12

Grout used to backfill casings must comply with section 50-1.02C, except:

- 1. Grout must consist of cementitious material and water, and may contain an admixture if authorized. Cementitious material must comply with section 90-1.02B, except SCMs are not required. The minimum cementitious material content of the grout must not be less than 845 lb/cu yd of grout.
- 2. Aggregate must be used to extend the grout as follows:

- 2.1. Aggregate must consist of at least 70 percent fine aggregate and approximately 30 percent pea gravel, by weight.
- 2.2. Fine aggregate must comply with section 90-1.02C(3).
- 2.3. Size of pea gravel must be such that 100 percent passes the 1/2-inch sieve, at least 90 percent passes the 3/8-inch sieve, and not more than 5 percent passes the no. 8 sieve.
- 3. California Test 541 is not required.
- 4. Grout is not required to pass through a sieve with a 0.07-inch maximum clear opening before being introduced into the grout pump.

Replace section 49-3.02B(8) with:

01-20-12

49-3.02B(8) Spacers

Spacers must comply with section 52-1.03D, except you may use plastic spacers.

Plastic spacers must:

- 1. Comply with sections 3.4 and 3.5 of the Concrete Reinforcing Steel Institute's *Manual of Standard Practice*
- 2. Have at least 25 percent of their gross plane area perforated to compensate for the difference in the coefficient of thermal expansion between the plastic and concrete
- 3. Be of commercial quality

Add to section 49-3.02C(4):

01-20-12

Unless otherwise shown, the bar reinforcing steel cage must have at least 3 inches of clear cover measured from the outside of the cage to the sides of the hole or casing.

Place spacers at least 5 inches clear from any inspection tubes.

Place plastic spacers around the circumference of the cage and at intervals along the length of the cage, as recommended by the manufacturer.

^^^^^^

50 PRESTRESSING CONCRETE

01-18-13

Replace the 3rd paragraph of section 50-1.01D(2) with:

10-19-12

The Department may verify the prestressing force using the Department's load cells.

Replace the 6th paragraph of section 50-1.01D(3) with:

01-18-13

Jacking equipment must be calibrated as follows:

- 1. Each jack and its gage must be calibrated as a unit.
- 2. Each jack used to tension prestressing steel permanently anchored at 25 percent or more of its specified minimum ultimate tensile strength must be calibrated by METS within 1 year of use and after each repair. You must:
 - 2.1. Schedule the calibration of the jacking equipment with METS
 - 2.2. Verify that the jack and supporting systems are complete, with proper components, and are in good operating condition

- 2.3. Mechanically calibrate the gages with a dead weight tester or other authorized means before calibration of the jacking equipment by METS
- 2.4. Provide enough labor, equipment, and material to (1) install and support the jacking and calibration equipment and (2) remove the equipment after the calibration is complete
- 2.5. Plot the calibration results
- 3. Each jack used to tension prestressing steel permanently anchored at less than 25 percent of its specified minimum ultimate tensile strength must be calibrated by an authorized laboratory within 6 months of use and after each repair.

Replace "diameter" in item 9 in the list in the 1st paragraph of section 50-1.02D with:

04-20-12

cross-sectional area

Add to section 50-1.02:

09-16-11

50-1.02G Sheathing

Sheathing for debonding prestressing strand must:

- 1. Be split or un-split flexible polymer plastic tubing
- 2. Have a minimum wall thickness of 0.025 inch
- 3. Have an inside diameter exceeding the maximum outside diameter of the strand by 0.025 to 0.14 inch

Split sheathing must overlap at least 3/8 inch.

Waterproofing tape used to seal the ends of the sheathing must be flexible adhesive tape.

The sheathing and waterproof tape must not react with the concrete, coating, or steel.

Add to section 50-1.03B(1):

01-20-12

After seating, the maximum tensile stress in the prestressing steel must not exceed 75 percent of the minimum ultimate tensile strength shown.

Add to section 50-1.03B(2):

09-16-11

50-1.03B(2)(e) Debonding Prestressing Strands

Where shown, debond prestressing strands by encasing the strands in plastic sheathing along the entire length shown and sealing the ends of the sheathing with waterproof tape.

Distribute the debonded strands symmetrically about the vertical centerline of the girder. The debonded lengths of pairs of strands must be equal.

Do not terminate debonding at any one cross section of the member for more than 40 percent of the debonded strands or 4 strands, whichever is greater.

Thoroughly seal the ends with waterproof tape to prevent the intrusion of water or cement paste before placing the concrete.

51 CONCRETE STRUCTURES

10-19-12

Replace the paragraphs of section 51-1.01A with:

10-19-12

Section 51-1 includes general specifications for constructing concrete structures.

Earthwork for the following concrete structures must comply with section 19-3:

- 1. Sound wall footings
- 2. Sound wall pile caps
- 3. Culverts
- 4. Barrier slabs
- 5. Junction structures
- 6. Minor structures
- 7. Pipe culvert headwalls, endwalls, and wingwalls for a pipe with a diameter of 5 feet or greater

Falsework must comply with section 48-2.

Joints must comply with section 51-2.

Elastomeric bearing pads must comply with section 51-3.

Reinforcement for the following concrete structures must comply with section 52:

- 1. Sound wall footings
- 2. Sound wall pile caps
- 3. Barrier slabs
- 4. Junction structures
- 5. Minor structures
- 6. PC concrete members

You may use RSC for a concrete structure only where the specifications allow the use of RSC.

Add to section 51-1.03C(2)(c)(i):

04-20-12

Permanent steel deck forms are only allowed where shown or if specified as an option in the special provisions.

Replace the 3rd paragraph of section 51-1.03C(2)(c)(ii) with:

04-20-12

Compute the physical design properties under AISI's North American Specification for the Design of Cold-Formed Steel Structural Members.

Replace the 8th paragraph of section 51-1.03D(1) with:

10-19-12

Except for concrete placed as pipe culvert headwalls and endwalls, slope paving and aprons, and concrete placed under water, consolidate concrete using high-frequency internal vibrators within 15 minutes of placing concrete in the forms. Do not attach vibrators to or hold them against forms or reinforcing steel. Do not displace reinforcement, ducts, or prestressing steel during vibrating.

Add to section 51-1.03E(5):

08-05-11

Drill the holes without damaging the adjacent concrete. If reinforcement is encountered during drilling before the specified depth is attained, notify the Engineer. Unless coring through the reinforcement is authorized, drill a new hole adjacent to the rejected hole to the depth shown.

Replace "Reserved" in section 51-1.03F(5)(b) with:

04-20-12

51-1.03F(5)(b)(i) General

Except for bridge widenings, texture the bridge deck surfaces longitudinally by grinding and grooving or by longitudinal tining.

10-19-12

For bridge widenings, texture the deck surface longitudinally by longitudinal tining.

04-20-12

In freeze-thaw areas, do not texture PCC surfaces of bridge decks.

51-1.03F(5)(b)(ii) Grinding and Grooving

When texturing the deck surface by grinding and grooving, place a 1/4 inch of sacrificial concrete cover on the bridge deck above the finished grade shown. Place items to be embedded in the concrete based on the final profile grade elevations shown. Construct joint seals after completing the grinding and grooving.

Before grinding and grooving, deck surfaces must comply with the smoothness and deck crack treatment requirements.

Grind and groove the deck surface as follows:

- 1. Grind the surface to within 18 inches of the toe of the barrier under section 42-3. Grinding must not reduce the concrete cover on reinforcing steel to less than 1-3/4 inches.
- 2. Groove the ground surfaces longitudinally under section 42-2. The grooves must be parallel to the centerline.

51-1.03F(5)(b)(iii) Longitudinal Tining

When texturing the deck surface by longitudinal tining, perform initial texturing with a burlap drag or broom device that produces striations parallel to the centerline. Perform final texturing with spring steel tines that produce grooves parallel with the centerline.

The tines must:

- 1. Be rectangular in cross section
- 2. Be from 3/32 to 1/8 inch wide on 3/4-inch centers
- 3. Have enough length, thickness, and resilience to form grooves approximately 3/16 inch deep

Construct grooves to within 6 inches of the layout line of the concrete barrier toe. Grooves must be from 1/8 to 3/16 inch deep and 3/16 inch wide after concrete has hardened.

For irregular areas and areas inaccessible to the grooving machine, you may hand construct grooves. Hand-constructed grooves must comply with the specifications for machine-constructed grooves.

Tining must not cause tearing of the deck surface or visible separation of coarse aggregate at the surface.

Replace the paragraphs of section 51-1.04 with:

10-19-12

If concrete involved in bridge work is not designated by type and is not otherwise paid for under a separate bid item, the concrete is paid for as structural concrete, bridge.

The payment quantity for structural concrete includes the volume in the concrete occupied by bar reinforcing steel, structural steel, prestressing steel materials, and piling.

The payment quantity for seal course concrete is the actual volume of seal course concrete placed except the payment quantity must not exceed the volume of concrete contained between vertical planes 1 foot outside the neat lines of the seal course shown. The Department does not adjust the unit price for an increase or decrease in the seal course concrete quantity.

Structural concrete for pier columns is measured as follows:

- 1. Horizontal limits are vertical planes at the neat lines of the pier column shown.
- 2. Bottom limit is the bottom of the foundation excavation in the completed work.
- 3. Upper limit is the top of the pier column concrete shown.

The payment quantity for drill and bond dowel is determined from the number and depths of the holes shown.

Replace "SSPC-QP 3" in the 1st paragraph of section 51-2.02A(2) with:

10-19-12

AISC-420-10/SSPC-QP 3

Replace the 2nd and 3rd paragraphs of section 51-2.02B(3)(b) with:

04-20-12

Concrete saws for cutting grooves in the concrete must have diamond blades with a minimum thickness of 3/16 inch. Cut both sides of the groove simultaneously for a minimum 1st pass depth of 2 inches. The completed groove must have:

- 1. Top width within 1/8 inch of the width shown or ordered
- 2. Bottom width not varying from the top width by more than 1/16 inch for each 2 inches of depth
- 3. Uniform width and depth

Cutting grooves in existing decks includes cutting any conflicting reinforcing steel.

Replace the 2nd paragraph of section 51-2.02E(1)(e) with:

08-05-11

Except for components in contact with the tires, the design loading must be the AASHTO LRFD Bridge Design Specifications Design Truck with 100 percent dynamic load allowance. Each component in contact with the tires must support a minimum of 80 percent of the AASHTO LRFD Bridge Design Specifications Design Truck with 100 percent dynamic load allowance. The tire contact area must be 10 inches measured normal to the longitudinal assembly axis by 20 inches wide. The assembly must provide a smooth-riding joint without slapping of components or tire rumble.

Add between the 1st and 2nd paragraphs of section 51-4.01A:

10-19-12

Prestressing concrete members must comply with section 50.

04-20-12

Delete the 2nd paragraph of section 51-4.01A.

Replace the 3rd paragraph of section 51-4.01C(2) with:

04-20-12

For segmental or spliced-girder construction, shop drawings must include the following additional information:

- 1. Details showing construction joints or closure joints
- 2. Arrangement of bar reinforcing steel, prestressing tendons, and pressure-grouting pipe
- 3. Materials and methods for making closures
- 4. Construction joint keys and surface treatment
- 5. Other requested information

For segmental girder construction, shop drawings must include concrete form and casting details.

10-19-12

Delete the 1st and 2nd paragraphs of section 51-4.02A.

Replace the 3rd paragraph of section 51-4.02B(2) with:

04-20-12

For segmental or spliced-girder construction, materials for construction joints or closure joints at exterior girders must match the color and texture of the adjoining concrete.

Add to section 51-4.02B(2):

04-20-12

At spliced-girder closure joints:

- 1. If shear keys are not shown, the vertical surfaces of the girder segment ends must be given a coarse texture as specified for the top surface of PC members.
- 2. Post-tensioning ducts must extend out of the vertical surface of the girder segment closure end sufficiently to facilitate splicing of the duct.

For spliced girders, pretension strand extending from the closure end of the girder segment to be embedded in the closure joint must be free of mortar, oil, dirt, excessive mill scale and scabby rust, and other coatings that would destroy or reduce the bond.

Add to section 51-4.03B:

04-20-12

The specifications for prestressing force distribution and sequencing of stressing in the post-tensioning activity in 50-1.03B(2)(a) do not apply if post-tensioning of spliced girders before starting deck construction is described. The composite deck-girder structure must be post-tensioned in a subsequent stage.

Temporary spliced-girder supports must comply with the specifications for falsework in section 48-2.

Before post-tensioning of spliced girders, remove the forms at CIP concrete closures and intermediate diaphragms to allow inspection for concrete consolidation.

Add between the 1st and 2nd paragraphs of section 51-7.01A:

10-19-12

Minor structures include:

- 1. Pipe culvert headwalls and endwalls for a pipe with a diameter less than 5 feet
- 2. Drainage inlets
- 3. Other structures described as minor structures

10-19-12

Delete the 4th paragraph of section 51-7.01A.

Replace the 1st and 2nd paragraphs of section 51-7.01B with:

10-19-12

Concrete must comply with the specifications for minor concrete.

Add to section 51:

10-19-12

51-8-51-15 RESERVED

^^^^^^

52 REINFORCEMENT

01-18-13
Add to section 52-1.01A:

07-20-12

Splicing of bar reinforcement must comply with section 52-6.

Replace the 1st and 2nd paragraphs of section 52-1.02B with:

10-19-12

Reinforcing bars must be deformed bars complying with ASTM A 706/A 706M, Grade 60, except you may use:

- 1. Deformed bars complying with ASTM A 615/A 615M, Grade 60, in:
 - 1.1. Junction structures
 - 1.2. Sign and signal foundations
 - 1.3. Minor structures
 - 1.4. Concrete crib members
 - 1.5. Mechanically-stabilized-embankment concrete panels
 - 1.6. Masonry block sound walls
- 2. Deformed or plain bars complying with ASTM A 615/A 615M, Grade 40 or 60, in:
 - 2.1. Slope and channel paving
 - 2.2. Concrete barriers Type 50 and 60
- 3. Plain bars for spiral or hoop reinforcement in structures and concrete piles

Add to the list in the 3rd paragraph of section 52-1.02B:

04-20-12

9. Shear reinforcement stirrups in PC girders

Replace the 6th paragraph of section 52-6.01D(4)(a) with:

01-18-13

Before performing service splice or ultimate butt splice testing, perform total slip testing on the service splice or ultimate butt splice test samples under section 52-6.01D(4)(b).

Replace section 52-6.02D with:

10-21-11

52-6.02D Ultimate Butt Splice Requirements

When tested under California Test 670, ultimate butt splice test samples must demonstrate necking as either of the following:

- 1. For "Necking (Option I)," the test sample must rupture in the reinforcing bar outside of the affected zone and show visible necking.
- 2. For "Necking (Option II)," the largest measured strain must be at least:
 - 2.1. Six percent for no. 11 and larger bars
 - 2.2. Nine percent for no. 10 and smaller bars

Replace the 2nd and 3rd paragraphs of section 52-6.03B with:

01-18-13

Do not splice the following by lapping:

- 1. No. 14 bars
- 2. No. 18 bars
- 3. Hoops
- 4. Reinforcing bars where you cannot provide a minimum clear distance of 2 inches between the splice and the nearest adjacent bar

^^^^^^

54 WATERPROOFING

04-20-12

Add between "be" and "3/8 inch" in the 3rd paragraph of section 54-4.02C:

at least

^^^^^

56 SIGNS

07-20-12

Delete item 2 in the list in the 4th paragraph of section 56-3.01A.

07-20-12

04-20-12

Delete the 7th paragraph of section 56-3.02K(2).

07-20-12

Delete item 4 in the list in the 1st paragraph of section 56-3.02M(1).

07-20-12

Delete "and box beam-closed truss" in the 2nd paragraph of section 56-3.02M(3)(a).

07-20-12

^^^^^^

57 WOOD AND PLASTIC LUMBER STRUCTURES

10-19-12

Replace "51-2.01C(3)" in the 1st paragraph of section 57-2.01C(3)(a) with:

57-2.01C(3)

10-19-12

^^^^^^

58 SOUND WALLS

10-19-12

Delete the 3rd paragraph of section 58-1.01.

10-19-12

Replace the 1st paragraph of section 58-2.01D(5)(a) with:

08-05-11

You must employ a special inspector and an authorized laboratory to perform Level 1 inspections and structural tests of masonry to verify the masonry construction complies with section 1704, "Special Inspections," and section 2105, "Quality Assurance," of the 2007 CBC.

10-19-12

Delete the 1st paragraph of section 58-2.02F.

^^^^^^

59 PAINTING

10-19-12

Replace "SSPC-SP 10" at each occurrence in section 59 with:

SSPC-SP 10/NACE no. 2

10-19-12

Replace "SSPC-SP 6" at each occurrence in section 59 with:

10-19-12

SSPC-SP 6/NACE no. 3

Replace "SSPC-CS 23.00" at each occurrence in section 59 with:

10-19-12

SSPC-CS 23.00/AWS C 2.23M/NACE no. 12

Replace "SSPC-QP 3 or AISC SPE, Certification P-1 Enclosed" in item 3 in the list in the 1st paragraph of section 59-2.01D(1) with:

10-19-12

AISC-420-10/SSPC-QP 3 (Enclosed Shop)

Replace the paragraphs in section 59-2.03A with:

10-19-12

Clean and paint all exposed structural steel and other metal surfaces.

You must provide enclosures for cleaning and painting structural steel. Cleaning and painting of new structural steel must be performed in an Enclosed Shop as defined in AISC-420-10/SSPC-QP 3. Maintain atmospheric conditions inside enclosures within specified limits.

Except for blast cleaning within closed buildings, perform blast cleaning and painting during daylight hours.

Add to section 59-2.03C:

10-19-12

59-2.03C(3) Moisture-Cured Polyurethane Coating Reserved

Replace item 1 in the list in the 2nd paragraph of section 59-2.03C(1) with:

10-19-12

1. Apply a stripe coat of undercoat paint on all edges, corners, seams, crevices, interior angles, junctions of joining members, weld lines, and similar surface irregularities. The stripe coat must completely hide the surface being covered. If spot blast cleaning portions of the bridge, apply the stripe coat of undercoat paint before each undercoat and follow with the undercoat as soon as practical. If removing all existing paint from the bridge, apply the undercoat first as soon as practical and follow with the stripe coat of undercoat paint for each undercoat.

Add to section 59-2.03C(2)(a):

10-19-12

Coatings for new structural steel must comply with the requirements shown in the following table:

Zinc Coating System for New Structural Steel

Description	Coating	Dry film thickness (mils)			
All surfaces:					
Undercoat	Inorganic zinc primer, AASHTO M 300 Type I or II	4–8			
Finish coat ^a	Exterior grade latex, 2 coats	2 minimum each coat, 4–8 total			
Total thickness, all coats		8–14			

^aIf no finish coats are described, a final coat of inorganic zinc primer is required

Coatings for existing structural steel must comply with the requirements shown in the following table:

85 of 95

Zinc Coating System for Existing Structural Steel

Description	Coating	Dry film thickness (mils)
Connections to new		
structural steel: ^a		
Undercoat	Inorganic zinc primer,	4–8
	AASHTO M 300 Type I or II	
Finish coat ^b	Exterior grade latex,	2 minimum each coat,
	2 coats	4–8 total
Total thickness, all coats		8–14
Other surfaces cleaned to		
bare metal:		
1st undercoat	State Specification PWB 145	2–3
2nd undercoat	State Specification PWB 146	2–3
1st finish coat	State Specification PWB 171	1.5–3
2nd finish coat	State Specification PWB 172	1.5–3
Total thickness, all coats		7–12
Existing painted surfaces		
to be topcoated:		
Undercoat	State Specification PWB 146	2–3
1st finish coat	State Specification PWB 171	1.5–3
2nd finish coat	State Specification PWB 172	1.5–3
Total thickness, new coats		5–9

^aIncludes the following locations:

- 1. New and existing contact surfaces
- 2. Existing member surfaces under HS bolt heads, nuts, or washers
- 3. Bare surfaces of existing steel after trimming, cutting, drilling, or reaming
- 4. Areas within a 4-inch radius from the point of application of heat for welding or flame cutting

07-20-12

Delete "and box beam-closed truss" in the 1st sentence in the 1st paragraph of section 59-5.03.

DIVISION VII DRAINAGE 62 ALTERNATIVE CULVERTS

10-19-12

Add to the end of section 62-1.01:

10-19-12

Alternative culverts include concrete collars and concrete tees and reinforcement for connecting new pipe to existing or new facilities. Concrete for the collars and tees must be minor concrete. Reinforcement for the concrete collars or tee connections must comply with section 52.

^^^^^^

^bIf no finish coats are described, a final coat of inorganic zinc primer is required

64 PLASTIC PIPE

10-19-12

Replace the 2nd paragraph of section 64-1.01A with:

10-19-12

Plastic pipe includes all necessary elbows, wyes, tees, other branches, fittings, coupling systems, concrete collars or tees, and reinforcement.

^^^^^^

65 CONCRETE PIPE

10-19-12

Replace the 2nd paragraph of section 65-1.01 with:

10-19-12

Concrete pipe includes all necessary elbows, wyes, tees, other branches, concrete collars or tees, and reinforcement.

^^^^^^

70 MISCELLANEOUS DRAINAGE FACILITIES

01-18-13 **Replace section 70-5.02A(2) with:**

01-20-12

70-5.02A(2) Plastic Flared End Sections

Plastic flared end sections must comply with ASTM D 3350.

Replace the 2nd, 3rd, and 4th paragraphs of section 70-7.02B with:

01-18-13

Before shipping, the exterior surfaces of the casing must be cleaned, primed, and coated to comply with ANSI/AWWA C213 or ANSI/AWWA C214.

Wrapping tape for repairing damaged coating and wrapping field joints and fittings must be a pressuresensitive PVC or polyethylene tape with a minimum thickness of 50 mils, 2 inches wide.

Add to section 70-7.03:

01-18-13

Repair damaged coating on the casing and wrap field joints and fittings with wrapping tape as follows:

- 1. Before wrapping, thoroughly clean and prime the pipe casing, joints, and fittings under the tape manufacturer's instructions.
- Wrap the tape tightly with 1/2 uniform lap, free from wrinkles and voids to provide not less than a 100mil thickness.
- 3. Wrapping at joints must extend at least 6 inches over adjacent pipe casing coverings. Apply tension such that the tape will conform closely to contours of the joint.

^^^^^

DIVISION VIII MISCELLANEOUS CONSTRUCTION 72 SLOPE PROTECTION

01-18-13

Replace the row under "Class" in the table in the 1st paragraph of section 72-3.02B with:

						01-20-12
Γ	1/2 T	1/4 T	Light	Facing	Cobble	

Replace the row under "Rock class" in the table in the 2nd paragraph of section 72-3.03E with:

					01-20-12
1/2 T	1/4 T	Light	Facing	Cobble	

Add to section 72-11.01B:

01-18-13

Expanded polystyrene and premolded expansion joint filler must comply with section 51-2.

Replace the 1st paragraph of section 72-11.01C(2) with:

01-18-13

Construct and finish minor concrete slope paving under section 51-1.

^^^^^^

74 PUMPING EQUIPMENT AND CONTROLS

01-20-12

Replace the 1st sentence of the 1st paragraph in section 74-2.01D(2) with:

01-20-12

Drainage pumps must be factory certified under ANSI/HI 14.6.

^^^^^

75 MISCELLANEOUS METAL

10-19-12

Replace "SSPC-QP 3" in the 3rd paragraph of section 75-1.03E(4) with:

10-19-12

AISC-420-10/SSPC-QP3

^^^^^^

Replace section 78 with:

07-20-12

78 INCIDENTAL CONSTRUCTION

07-20-12 **78-1 GENERAL**

Section 78 includes specifications for incidental bid items that are not closely associated with other sections.

78-2-78-50 RESERVED

^^^^^^

80 FENCES

10-19-12 **Add to section 80-2.02D:**

Vertical stays must:

- 1. Comply with ASTM A641
- 2. Be 12-1/2 gage
- 3. Have a Class 3 zinc coating

Replace item 1 in the list in section 80-2.02E with:

10-19-12

10-19-12

Comply with ASTM A 116, Type Z, Grade 60, Class 1

Add after "galvanized wire" in the 1st paragraph of section 80-2.02F:

10-19-12

complying with ASTM A 641

Replace the 3rd and 4th paragraphs of section 80-2.02F with:

10-19-12

Each staple used to fasten barbed wire and wire mesh fabric to wood posts must:

- 1. Comply with ASTM F 1667
- 2. Be at least 1-3/4 inches long
- 3. Be manufactured from 9-gage galvanized wire

Wire ties used to fasten barbed wire and wire mesh to metal posts must be at least 11-gage galvanized wire complying with ASTM F 626. Clips and hog rings used for metal posts must be at least 9-gage galvanized wire complying with ASTM F 626.

Replace the 8th through 14th paragraphs of section 80-2.03 with:

10-19-12

Attach the wire mesh and barbed wire to each post.

Securely fasten tension wires to wood posts. Make a single or double loop around each post at each attachment point and staple the wire to the post. Use wire ties, hog rings, or wire clips to fasten the wires to the metal posts.

Connect each wood brace to its adjacent post with a 3/8 by 4-inch steel dowel. Twist the tension wires until the installation is rigid.

Stretch barbed wire and wire mesh fabric and fasten to each wood or steel end, corner, or gate post. Apply tension according to the manufacturer's instructions using a mechanical stretcher or other device designed for such use. If no tension is specified by the manufacturer, use 250 pounds for the required tension. Evenly distribute the pull over the longitudinal wires in the wire mesh such that no more than 50 percent of the original depth of the tension curves is removed. Do not use a motorized vehicle, truck, or tractor to stretch the wire.

Attach barbed wire and wire mesh fabric to the private-property side of posts. On curved alignments, place the wire mesh and barbed wire on the face of the post against which the normal pull of the wire mesh and wire will be exerted. Terminate the wire mesh and barbed wire at each end, corner, pull, and gate post in the new fence line. Attach wire mesh and barbed wire to each wood or steel end, corner, pull, or gate post by wrapping each horizontal strand around the post and tying it back on itself with at least 4 tightly-wound wraps.

At line posts, fasten the wire mesh to the post at the top and bottom and at intermediate points not exceeding 10 inches apart. Fasten each line of barbed wire to each line post. Use wire ties or clips to fasten the wires to metal posts under the post manufacturer's instructions. Drive staples crosswise with the grain of the wood and pointed slightly downward. Drive staples just short of actual contact with the wires to allow free longitudinal movement of those wires and to prevent damage to the wire's protective coating. Secure all wires to posts to maintain horizontal alignment.

Splices in barbed wire and wire mesh are allowed provided there are no more than 2 splices per 50 feet of fence. Use commercially-available galvanized mechanical wire splices or a wire splice created by tying off wire. Install mechanical wire splices with a tool designed for that purpose under the manufacturer's instructions. Tie off the wire as follows:

- 1. Carry the ends of each wire 3 inches past the tied-off knot location and wrap around the wire for at least 6 turns in opposite directions.
- 2. Remove the splice tool and close the space by pulling the end of the wires together.
- 3. Cut the unused ends of the wire close and neat.

Add to "≤ 6" in the table in the 4th paragraph of section 80-3.02B:

10-19-12

feet

DIVISION IX TRAFFIC CONTROL FACILITIES 83 RAILINGS AND BARRIERS

10-19-12

Replace "80-2.02" in the 2nd paragraph of section 83-1.02E with:

10-19-12

80-3.02B

Add to section 83-2.02D(1):

10-21-11

For a concrete barrier transition:

- 1. Remove portions of the existing concrete barrier where shown under section 15-3
- 2. Roughen the contact surface of the existing concrete barrier
- 3. Drill and bond dowels into the existing concrete barrier under section 51-1

Add to section 83-2.02:

10-19-12

83-2.02H-83-2.02M Reserved

84 TRAFFIC STRIPES AND PAVEMENT MARKINGS

01-20-12

Replace the 1st paragraph in section 84-2.04 with:

01-20-12

A double extruded thermoplastic traffic stripe consisting of two 4-inch wide yellow stripes is measured as 2 traffic stripes.

A double sprayable thermoplastic traffic stripe consisting of two 4-inch wide yellow stripes is measured as 1 traffic stripe.

Add to section 84:

01-20-12

84-6 THERMOPLASTIC TRAFFIC STRIPES AND PAVEMENT MARKINGS WITH ENHANCED WET NIGHT VISIBILITY

Reserved

84-7-84-10 RESERVED

86 ELECTRICAL SYSTEMS

10-19-12

Replace section 86-2.06 with:

01-20-12

86-2.06 PULL BOXES 86-2.06A General 86-2.06A(1) Cover Marking

Marking must be clearly defined, uniform in depth, and parallel to either the long or short sides of the cover.

Marking letters must be 1 to 3 inches high.

Before galvanizing steel or cast iron cover, apply marking by one of the following methods:

- 1. Use cast iron strip at least 1/4 inch thick with letters raised a minimum of 1/16 inch. Fasten strip to cover with 1/4-inch flathead stainless steel machine bolts and nuts. Peen bolts after tightening.
- 2. Use sheet steel strip at least 0.027 inch thick with letters raised a minimum of 1/16 inch. Fasten strip to cover by spot welding, tack welding, or brazing, with 1/4-inch stainless steel rivets or 1/4-inch roundhead stainless steel machine bolts and nuts. Peen bolts after tightening.
- 3. Bead weld the letters on cover such that the letters are raised a minimum of 3/32 inch.

86-2.06A(2) Installation and Use

Space pull boxes no more than 200 feet apart. You may install additional pull boxes to facilitate the work.

You may use a larger standard size pull box than that shown on the plans or specified.

A pull box in ground or sidewalk area must be installed as follows:

- 1. Embed bottom of the pull box in crushed rock.
- 2. Place a layer of roofing paper on the crushed rock.
- 3. Place grout over the layer of roofing paper. Grout must be 0.50 to 1 inch thick and sloped toward the drain hole.
- 4. Make a 1-inch drain hole in the center of the pull box through the grout and roofing paper.
- 5. Place grout between the pull box and the pull box extension, and around conduits.

The top of the pull box must be flush with the surrounding grade or the top of an adjacent curb, except in unpaved areas where the pull box is not immediately adjacent to and protected by a concrete foundation, pole, or other protective construction. Place the pull box 1-1/4 inches above the surrounding grade. Where practical, place a pull box shown in the vicinity of curbs or adjacent to a standard on the side of the foundation facing away from traffic. If a pull box is installed in a sidewalk area, adjust the depth of the pull box so that the top of the pull box is flush with the sidewalk.

Reconstruct the sump of an existing pull box if disturbed by your activities. Remove old grout and replace with new if the sump was grouted.

86-2.06B Non-Traffic-Rated Pull Boxes

Reserved

86-2.06C Traffic Pull Boxes

Traffic pull box and cover must comply with ASTM C857, "Standard Practice for Minimum Structural Design Loading for Underground Precast Concrete Utility Structures," for HS20-44 loading. You must be able to place the load anywhere on the box and cover for 1 minute without causing cracks or permanent deformations.

Frame must be anchored to the box with 1/4 by 2-1/4 inch concrete anchors. Four concrete anchors must be included for No. 3-1/2(T) pull box; one placed in each corner. Six concrete anchors must be included for No. 5(T) and No. 6(T) pull boxes; one placed in each corner and one near the middle of each of the longer sides.

Nuts must be zinc-plated carbon steel, vibration resistant, and have a wedge ramp at the root of the thread.

After installation of traffic pull box, install the steel cover and keep it bolted down when your activities are not in progress at the pull box. When the steel cover is placed for the final time, the cover and Z bar frame must be cleaned of debris and tightened securely.

Steel cover must be countersunk approximately 1/4 inch to accommodate the bolt head. When tightened, the bolt head must not exceed more than 1/8 inch above the top of the cover.

Concrete placed around and under traffic pull boxes must be minor concrete.

Replace "project" in the 3rd paragraph of section 86-2.11A with:

10-19-12

work

Replace "Contract" in item 2 in the list in the 11th paragraph of section 86-2.11A with:

10-19-12

work

^^^^^^

88 GEOSYNTHETICS

01-18-13

Replace the row for hydraulic bursting strength in the table in the 2nd paragraph of section 88-1.02B with:

10-19-12

Puncture strength, lb min	ASTM D 6241	310
Trapezoid tearing strength, lb min	ASTM D 4533	56

Replace the 3rd paragraph in section 88-1.02C with:

10-19-12

Geocomposite wall drain must be from 0.25 to 2 inches thick.

Replace the value for permittivity of woven fabric in the table in the 1st paragraph of section 88-1.02E with:

01-20-12

0.05

Replace the value for apparent size opening of nonwoven fabric in the table in the 1st paragraph of section 88-1.02E with:

01-20-12

0.012

Replace the table in the 1st paragraph of section 88-1.02G with:

01-20-12

Sediment Filter Bag

Proporty	Test	Values		
Property	rest	Woven	Nonwoven	
Grab breaking load, lb, 1-inch grip min, in each direction	ASTM D 4632	200	250	
Apparent elongation, percent min, in each direction	ASTM D 4632	10	50	
Water flow rate, gal per minute/sq ft min and max average roll value	ASTM D 4491	100-200	75-200	
Permittivity, sec ⁻¹ min	ASTM D 4491	1.0	1.0	
Apparent opening size, inches max average roll value	ASTM D 4751	0.023	0.012	
Ultraviolet resistance, % min retained grab breaking load, 500 hr.	ASTM D 4355	70	70	

Replace the table in the 1st paragraph of section 88-1.02H with:

01-20-12

Temporary Cover

Proporty	Test	Values		
Property	rest	Woven	Nonwoven	
Grab breaking load, lb, 1-inch grip min, in each direction	ASTM D 4632	200	200	
Apparent elongation, percent min, in each direction	ASTM D 4632	15	50	
Water flow rate, gal per minute/sq ft min and max average roll value	ASTM D 4491	4-10	80-120	
Permittivity, sec ⁻¹ min	ASTM D 4491	0.05	1.0	
Apparent opening size, inches max average roll value	ASTM D 4751	0.023	0.012	
Ultraviolet resistance, % min retained grab breaking load, 500 hr.	ASTM D 4355	70	70	

Replace section 88-1.02P with:

01-18-13

88-1.02P Biaxial Geogrid

Geosynthetics used for biaxial geogrid must be a punched and drawn polypropylene material formed into an integrally formed biaxial grid. When tested under the referenced test methods, properties of biaxial geogrid must have the values shown in the following table:

Biaxial Geogrid

Property	Test	Value
Aperture size, inch ^a min and max	Calipered	0.8-1.3 x 1.0-1.6
Rib thickness, inch min	Calipered	0.04
Junction thickness, inch min	Calipered	0.150
Tensile strength, 2% strain, lb/ft ^a min	ASTM D 6637	410 x 620
Tensile strength at ultimate, lb/ft ^a min	ASTM D 6637	1,310 x 1,970
Ultraviolet resistance, percent min retained tensile strength, 500 hours	ASTM D 4355	100
Junction strength, lb/ft ^a min	ASTM D 7737	1,220 x 1,830
Overall flexural rigidity, mg-cm min	ASTM D 7748	750,000
Torsional rigidity at 20 cm-kg, mm-kg/deg ^b min	GRI:GG9	0.65

^aMachine direction x cross direction

^bGeosynthetic Research Institute, Test Method GG9, *Torsional Behavior of Bidirectional Geogrids When Subjected to In-Plane Rotation*

^^^^^^

DIVISION X MATERIALS 90 CONCRETE

08-05-11

Replace the 3rd paragraph of section 90-1.01C(7) with:

08-05-11

Submit weighmaster certificates in printed form or, if authorized, in electronic media. Present electronic media in a tab-delimited format on a CD or DVD. Captured data for the ingredients represented by each batch must be line feed carriage return and one line separate record with sufficient fields for the specified data.

Replace the 3rd paragraph of section 90-3.01C(5) with:

08-05-11

Production data must be input by hand into a pre-printed form or captured and printed by the proportioning device. Present electronic media containing recorded production data in a tab-delimited format on a CD or DVD. Each capture of production data must be followed by a line feed carriage return with sufficient fields for the specified data.

91 PAINT

10-19-12

Add to section 91-2:

10-19-12

91-2.03 MOISTURE-CURED POLYURETHANE COATING

Reserved

Replace "saint" in the 1st paragraph of section 91-4.05 with:

10-19-12

paint

92 ASPHALTS

^^^^^^

01-20-12

Replace the row for dynamic shear for original binder in the table in the 1st paragraph of section 92-1.02B with:

						01-2	20-12
Dynamic shear,							
Test temperature at 10	_						
rad/s, ℃	T 315	58	64	64	64	70	
min G*/sin(delta), kPa		1.00	1.00	1.00	1.00	1.00	
max G*/sin(delta), kPa		2.00	2.00	2.00	2.00	2.00	

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